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NOAA Technical Memorandum NWS HYDRO-31



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CATCHMENT MODELING AND INITIAL PARAMETER  
ESTIMATION FOR THE NATIONAL WEATHER  
SERVICE RIVER FORECAST SYSTEM

Office of Hydrology  
Washington, D.C.  
June 1976

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- NWS HYDRO 14 National Weather Service River Forecast System Forecast Procedures. December, 1972. COM-73-10517)

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Eugene L. Peck

Office of Hydrology  
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## Preface

The enclosed papers were prepared for the International Symposium and Workshop on the Application of Mathematical Models in Hydrology and Water Resources Systems held in Bratislava, Czechoslovakia, on 8-13 September 1975.

The papers are being published in this format because the distribution of the original reports was extremely limited. There is a need for this information to be available to potential users of the catchment model of the National Weather Service River Forecast System. This system comprises a number of hydrologic models which are being incorporated into an operational river forecasting program. The system is being implemented by the Hydrologic Services Division and the Hydrologic Research Laboratory of the Office of Hydrology.

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## CATCHMENT MODELING WITH THE UNITED STATES NATIONAL WEATHER SERVICE RIVER FORECAST SYSTEM

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**ABSTRACT.** The system (NWSRFS) of conceptual hydrologic models and other procedures, used in the operational river forecasting program of the United States National Weather Service, is briefly described. Complete information on the system as it existed in 1972 was published. However, since then the operational system has been expanded and revised frequently. Information on new procedures will be published in the technical literature.

A major revision has been made in the soil moisture accounting for the catchment model. The components for soil moisture accounting of the Sacramento Model have replaced those of the modified Stanford Model as used in the original system. The conceptual features and characteristics of the Sacramento Model are discussed. The demonstration in the workshop of this symposium will be limited to the catchment model.

## INTRODUCTION

In 1971, the United States National Weather Service decided to develop and publish the National Weather Service River Forecast System (NWSRFS) (NOAA, 1972). This system is a comprehensive collection of the latest hydrologic techniques and includes the basic hydrologic techniques needed by the NWS River Forecast Centers to perform their operational functions. Each technique has been developed and/or evaluated by the Hydrologic Research Laboratory of the National Weather Service. These hydrologic techniques include, but are not necessarily limited to, the following:

1. A catchment model which, through the use of soil moisture accounting formulations and the mathematical modeling of flow through and above the soil mantle and within the channel, convert moisture input (rainfall or snowmelt) to a hydrograph of channel discharge at the outlet of the catchment.
2. A mathematical model of the accumulation and ablation of snow.
3. Channel routing models which model the translation and attenuation of a flood wave as it moves between two points in a channel.
4. Techniques for modeling the areal distribution of precipitation, to be used for computing the moisture input to a catchment on the basis of point values measured at rain gauges.

In addition to the hydrologic techniques, the system includes three other categories of material.

A - Procedures for archiving, retrieving and processing the types of data needed to apply the system.

B - Methods needed to calibrate the various hydrologic techniques, that is, to evaluate the parameters to apply a hydrologic or hydraulic model to a specific location.

C - Computer programs necessary to execute the hydrologic techniques and support procedures described above, in both the development and operational modes.

The system was begun in 1971, along the lines described above and published as NOAA Technical Memorandum NWS HYDRO-14, National Weather Service River Forecast System Forecast Procedures. As originally published, the system included a modification of the Stanford Watershed Model IV, based on the work of Crawford and Linsley (1966).

The nature and concept of the system are such that it may be expected to be constantly changing. New hydrologic techniques become available from time to time and, if they are judged to be superior to those in the system, substitutions are made. Changes and increases in the needs of forecast users

may present a need for new hydrologic products and the techniques needed to produce them. Advances in computing equipment and/or changes in the equipment available to the service also require revisions to the computer programs.

### NWSRFS MODIFICATIONS

Additional procedures are being included in the NWSRFS to expand the flexibility of the system. A major change has been made in the basic soil moisture accounting. The soil moisture accounting system of the catchment model developed in the NWS Sacramento, California River Forecast Center by Burnash, et al. (1973), is now included in the system. The method employed includes a minor modification of the temporal distribution function from that described in the original Sacramento model.

### SOIL MOISTURE MODEL

The soil moisture models that have been used in NWSRFS have been conceptual in design. This resulted from a firm belief that a number of benefits accrue from a strong physical base. Some of these are:

1. The performance of the model in simulating the past is the only available objective measure of the model's ability to predict the future. It is, however, an indirect and imperfect measure. Where accurate simulation of the past has been attained, a high degree of conceptuality enhances the probability of adequately predicting future events. This is especially true in the case of extreme events involving values of variables not experienced in historical data, or, experienced values of the variables but in unexperienced combinations.
2. Models of this type are necessarily complex and involve a large number of parameters. The evaluation of parameter values for a specific catchment is a very serious problem, always involving a number of successive approximations. The chances of obtaining something close to the true values of the parameters are increased if the first approximation is reasonable. If the parameters have real physical meaning, good first approximations of their values may be inferred from streamflow records and various observable basin characteristics.
3. Parameters based on conceptual considerations can sometimes be subjectively altered to reflect changes made or to be made to the physical characteristics of the catchment thereby mitigating the need to wait for a new data base to be developed.
4. A conceptual model can be applied to problems other than discharge prediction. Some examples are, movement of pollutants through the soil mantle, water temperature prediction and determination, and prediction of soil moisture levels for agricultural purposes.

used in the computation varies between these limits as a function of the amount of water in storage.

Flow Components. The model recognizes and generates five components of flow:

1. Direct runoff, resulting from moisture input being applied to the variable impervious area.
2. Surface runoff. When moisture input is supplied at a rate faster than it can enter the upper zone, the excess appears as surface runoff.
3. Interflow, lateral drainage from upper zone free water.
4. Supplementary base flow, lateral drainage from lower zone supplementary free water.
5. Primary base flow, lateral drainage from lower zone primary free water.

Evapotranspiration. Evapotranspiration rates in the Sacramento model may be estimated from meteorological variables or from pan observations. Either day-by-day or long-term values may be used to derive the demand curve. The catchment evapotranspiration - demand curve is a product of the computed evaporation index and a seasonal adjustment curve. The seasonal adjustment curve reflects the state of the vegetation. The moisture accounting within the model applies the evapotranspiration loss, directly or indirectly, to the various storages and/or to the channel. The amount taken from each location in the model is determined by a hierarchy of priorities and is limited by the availability of the moisture as well as by the computed demand.

Computational Technique. The movement of moisture through the soil mantle is a continuous process. The rate of flow at various points varies with the rate of moisture supply and with the contents of various storages. This process is modeled by a quasi-linear, open form computation. A single time step computation of the drainage and percolation loop involves the implicit assumption that the movement of moisture during the time step is defined by the conditions at the beginning of the time step. Since this assumption is not valid, the resultant approximation can be made acceptable only by the use of a short time step. In the model, the length of the step is volume dependent. That is, it is selected in such a way that no more than 5 mm of water may be involved in any single execution of the computational loop. The 5 mm limit is arbitrary. It was selected by the originators as being small enough to logically fulfill its function, and not so small as to cause excessively long execution times on the computer (IBM 1130) which was used to develop the model. Sensitivity tests to determine the optimal size of this limit should have a dependency upon soil type. The current limit represents a compromise to eliminate the need for an additional parameter.

Parameters. The soil moisture accounting portion of the Sacramento model, exclusive of the evapotranspiration demand curve, involves seventeen parameters. The demand curve can be defined by a series of ordinates, twelve in number, or by a formula involving five parameters. The temporal distribution function, which converts runoff volumes to a discharge hydrograph, involves a unit hydrograph, and, in some applications, a channel routing function.

The original Sacramento model applied the unit hydrograph to only the upper three components of flow. The two lower zone components were added to the channel flow in the time period in which they were released from the lower zone. In the NWSRFS version, the unit hydrograph is applied to the sum of all five components.

The application of the model in the NWSRFS involves moisture input in 6-hour time periods, and computed 6-hour runoff volumes. The short, repetitive computational time step described above is a subdivision of the 6-hour period and has mathematical significance only. The computations are accumulated over a 6-hour period and applied to a unit hydrograph function representing a 6-hour duration event.

Calibration. A very difficult problem which always accompanies the use of a hydrologic model is that of calibration or "parameter optimization." A model is obviously useless if its parameters cannot be evaluated. Yet, the determination of the optimal values of fifteen to twenty interrelated parameters is a formidable task. The National Weather Service has used a combination of manual and automatic optimization techniques. The term "manual" refers here to a procedure in which subjective adjustments to various parameters are made on the basis of specific characteristics of the output of previous computer runs. Automatic techniques are those in which the computer itself adjusts parameters in a semi-random manner, based on changes in the value of a single numerical error function. The method used is an application of the "Pattern Search" technique described by Monro (1971).

There is no doubt that a good set of parameters can be obtained using only manual methods. However, the procedure is time consuming in terms of man-hours and requires a degree of interplay with the computer often not available from larger systems. In addition, the hydrologist performing the optimization must possess a considerable degree of skill acquired through experience with the model. Automatic methods, on the other hand, are fast and simple to use. Besides being expensive from a computer usage standpoint, they have some inherent disadvantages. Some of these are: complete dependency on one error function, failure to attain an optimal solution due to non-convexity of the response surface in the vicinity of the starting point, and failure to recognize the effect of perturbing a group of parameters simultaneously. At its worst, such a procedure can degenerate into pure curve fitting and produce a set of parameters which fit the calibration data reasonably well, but which are hydrologically unrealistic.

Experience in fitting the model to a large number of catchments under operational conditions indicates that the procedure should be one involving both manual and automatic fitting where the strong points of each compensate the weak points of the other. Generally, much more is achieved by fitting manually first, then using the automatic optimizer after a reasonable fit has been obtained.

Data requirements for the model are somewhat greater than for simpler "event" type models, since the model utilizes a continuous record rather than a fragmentary one covering selected periods.

The length of the data base required for adequate calibration depends on a number of factors including the hydro-climatic characteristics of the catchment and the amount of hydrologic activity during the period in question. Typically, however, it runs 8 to 10 years.

### COMPLETE NWSRFS

The National Weather Service River Forecast System is continually being updated and expanded. It contains many models and procedures including the catchment model. Routing and data handling and processing procedures required to adapt the system to a particular river basin are also included in the complete NWSRFS system.

The modular form of the NWSRFS permits the incorporation of additions and improvements with a minimum of programming effort. A snow accumulation and ablation model (Anderson 1973) has been added to the original system. Dynamic (implicit) routing techniques for use on major rivers where serious backwater problems are encountered due to interconnected river systems or tidal effects (Fread 1973) are being incorporated into the system.

It is not planned to publish the entire revised NWSRFS since it is an operational system and subject to frequent modifications. The complete system will be available only on the NOAA's central computer system for use by the NWS River Forecast Centers. However, information on new and revised techniques will continue to be published in the literature.

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CALIBRATION OF NATIONAL WEATHER SERVICE RIVER FORECAST SYSTEM:  
INITIALIZING PARAMETERS FOR THE CATCHMENT MODEL

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**ABSTRACT.** Use of the catchment model in the National Weather Service River Forecast System (NWSRFS) requires the determination of 16 model parameters. The calibration process is greatly enhanced if rational initial estimates of model parameters can be found. Techniques are developed to derive initial parameter estimates directly from the hydrometeorological data base of a catchment. The techniques utilize catchment maps, precipitation records, and streamflow records to estimate the magnitudes of soil moisture storage components and appropriate drainage coefficients. Step by step demonstrations of the estimation procedure are included. As an example, parameter estimates are obtained for simulation of the South Yamhill River near Whiteson, Oregon.

## INTRODUCTION

The soil moisture accounting program of the catchment model developed in the National Weather Service (NWS) Sacramento, California, River Forecast Center by Burnash, et al. (1973), is presently used in the National Weather Service River Forecast System (NWSRFS) (NOAA 1972). A general description of the model is given in the companion paper prepared for this workshop (Peck 1975). Figure 1 is a flow diagram illustrating the various paths water takes in the model. A listing of the NWSRFS subroutine for this model appears in appendix D.

Calibration of the catchment model requires determination of values for 16 parameters associated with soil moisture accounting. This section describes methods for determining initial parameter values. All the parameters are depicted in figure 1.

## REQUIREMENTS FOR HYDROGRAPH SIMULATION

Simulation required to test the validity of the soil moisture parameters involves three other elements. These are:

1. Mean Areal Precipitation (MAP). This includes all the techniques and procedures necessary to arrive at basinwide estimates of mean areal precipitation for use by the soil moisture accounting portion of NWSRFS. Included are methods for estimating missing precipitation amounts, distributing estimated or accumulated precipitation, and adjusting precipitation data for orographic and/or other effects. In basins in which snow occurs, input to the catchment program consists of the liquid water reaching the soil mantle from a combination of rainfall and snowmelt. The snowmelt may be either estimated or computed from the NWSRFS snow accumulation and ablation model (Anderson 1973).

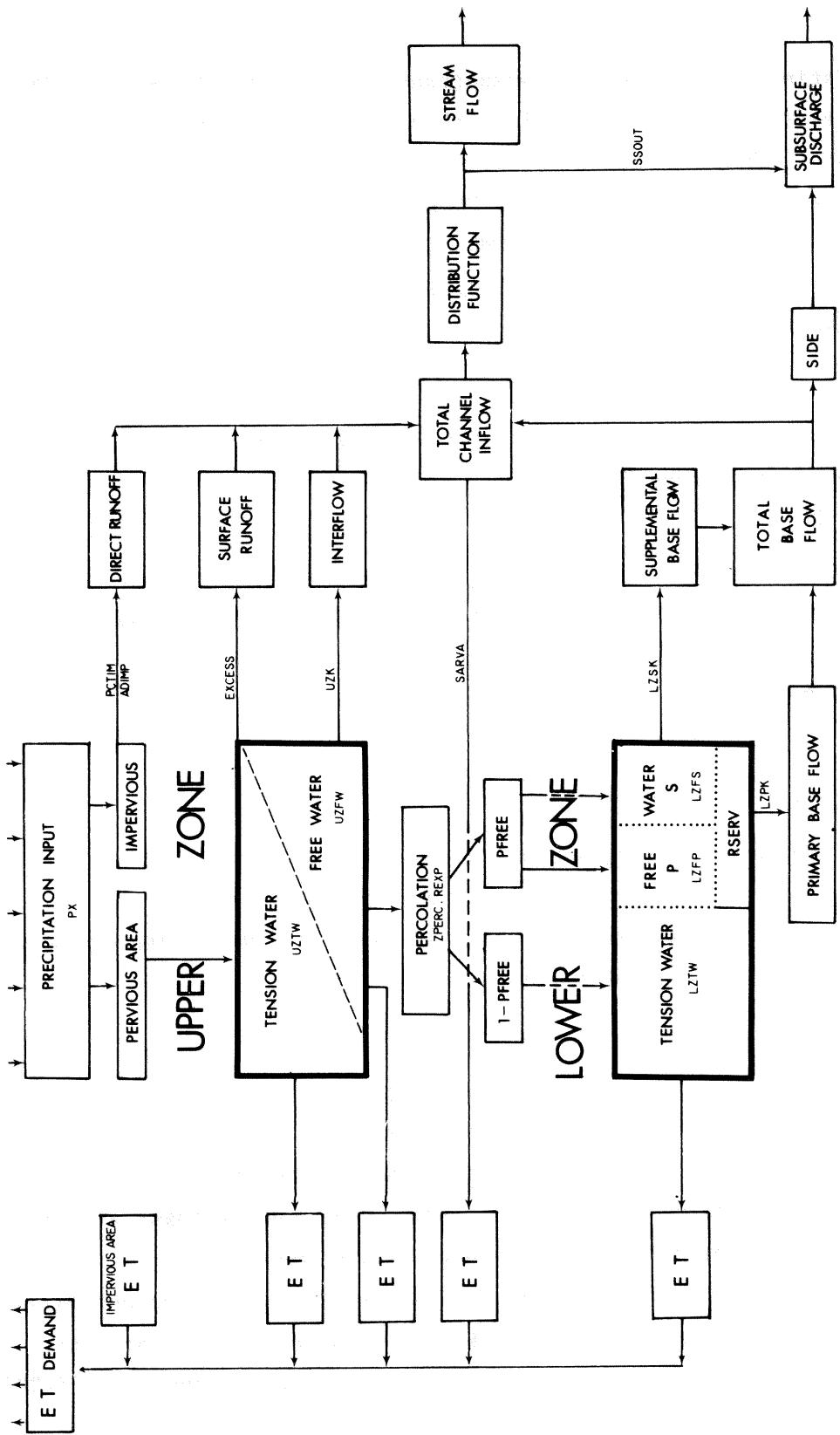


Figure 1.--NWSRFS catchment model (Sacramento).

2. Estimates of potential evapotranspiration for the basin. These values can be estimated from meteorological variables or from pan evaporation observations. They can be either day-by-day or long-term averages.

3. Channel routing function. This may be developed using standard unit hydrograph (UHG) techniques. The UHG can be used to determine a time-delay histogram which provides additional options for varying the shape of the routing function if required.

#### SOIL MOISTURE PARAMETERS

The parameters for the catchment model dealing with various phases of the soil moisture accounting are:

##### Direct runoff

PCTIM Fraction of impervious basin contiguous with stream channels.

ADIMP That fraction of the basin which becomes impervious as all tension water requirements are met.

SARVA Fraction of basin covered by streams, lakes and riparian vegetation.

##### Upper soil moisture zone

UZTWM Maximum capacity upper zone tension water in mm.

USFWM Maximum capacity of upper zone free water in mm.

UZK Lateral drainage rate of upper zone free water expressed as a fraction of contents per day.

##### Percolation

ZPERC A factor used to define the proportional increase in percolation from saturated to dry lower zone soil moisture conditions. This parameter indicates, when used with other parameters, the maximum percolation rate possible when upper zone storages are full and the lower zone soil moisture is 100% deficient.

REXP An exponent determining the rate of change of the percolation rate as the lower zone deficiency ratio varies from 1 to 0 (1 = completely dry; 0 = lower zone storage completely full).

Lower zone

- LZTWM Maximum capacity of lower zone tension water in mm.
- LZFSM Maximum capacity of lower zone supplemental free water storage in mm.
- LZSK Lateral drainage rate of lower zone supplemental free water expressed as a fraction of contents per day.
- LZFPM Maximum capacity of lower zone primary free water storage in mm.
- LZPK Lateral drainage rate of lower zone primary free water expressed as a fraction of contents per day.
- PFREE The percentage of percolation water which directly enters the lower zone free water without a prior claim by lower zone tension water.
- RSERV Fraction of lower zone free water not available for transpiration purposes (incapable of resupplying lower zone tension water).
- SIDE The ratio of unobserved to observed baseflow.
- SSOUT A fixed rate of discharge lost from the total channel flow.

## PARAMETER GROUPINGS

If the conceptual model is realistic for the basin, such that parameters have physical meaning, good first approximations for some of the parameters may be inferred from streamflow records, precipitation records, and other basin characteristics. The chances of obtaining the most representative set of parameters are increased with successive approximations if the first approximations are reasonable.

The soil moisture model parameters may be grouped according to the methods for obtaining first approximations. The parameters and their associated classifications are:

## 1. Parameters readily computed from observed hydrograph and precipitation

|       |       |
|-------|-------|
| LZFPM | LZSK  |
| LZPK  | PCTIM |
| LZFSM |       |

2. Parameters more difficult to estimate from observed hydrograph

|       |        |
|-------|--------|
| LZTWM | SSOUT  |
| UZTWM | UZFWM* |
| UZK   | PFREE* |

\*Relative size only.

3. Parameters estimated from maps of water area

SARVA

4. Relative values could possibly be estimated for the following parameters from soil percolation characteristics. However, the best first estimate is to use values from similar nearby basins that have been previously simulated.

ZPERC

REXP

5. Nominal starting values used

SIDE

ADIMP

RSERV

### INITIAL PARAMETER DETERMINATION

The South Yamhill River near Whiteson, Oregon, U.S.A., has been selected for use as an example for this workshop. Appendix A contains semilogarithmic plots of the observed hydrograph for this river for the water years 1963 (Oct. 1962 to Sept. 1963) and 1965 (Oct. 1964 to Sept. 1965). These plots contain sufficient variations in observed flows for computing those initial soil moisture values determined from observed hydrographs.

Hypothetical examples are discussed in this section to guide the workshop participant in selecting initial parameters for the South Yamhill Basin. For comparison purposes, actual examples of determination of initial parameter values for the South Yamhill Basin will be demonstrated in appendix B. The South Yamhill Basin was selected for an example since it has a large variation in hydrologic flow conditions, which makes it ideal for demonstrating determination of initial parameters.

Semilogarithmic hydrograph plots have commonly been used to separate hydrographs into principal flow components of surface runoff, interflow, and groundwater recession as shown in figure 2 (Linsley, Kohler, and Paulhus

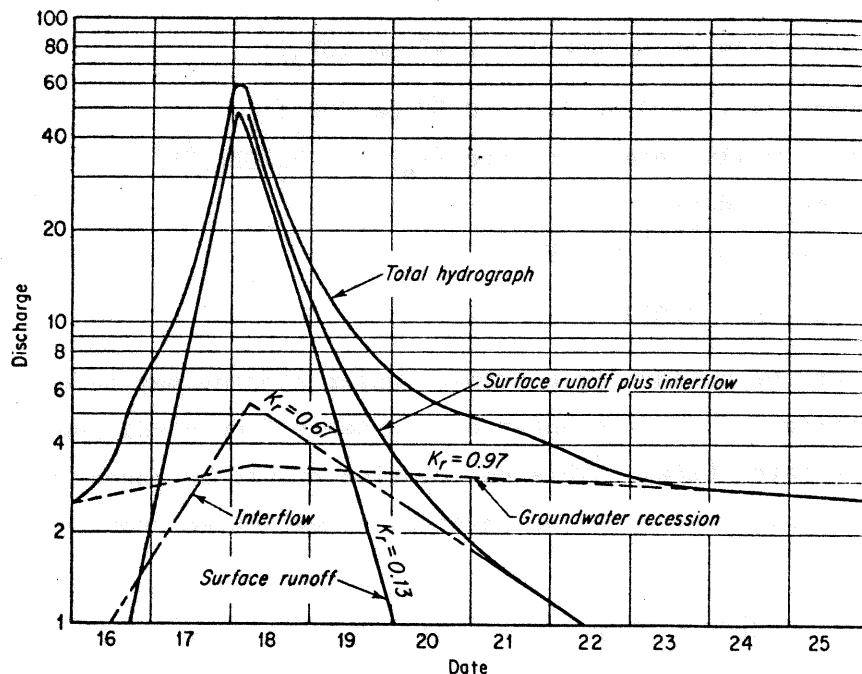


Figure 2.--Semilogarithmic plotting of a hydrograph, showing method of recession analysis.

1975). The characteristics of the hydrograph recession may be used to obtain initial values for the maximum capacities and depletion coefficients for the lower zone free water storages (LZFPM, LZFSM, LZPK, and LZSK).

If a groundwater recession continues for some time, the recession is characterized by two distinct slopes, with a much flatter recession occurring after a prolonged dry period. The developers of the soil moisture model believe the base flow can be modeled with two slopes representing two separate sources of base flow with separate exponential decaying functions. For the model being used, these are the supplemental and primary free water storages of the lower zone. Analyses of the recession provide methods for estimating the depletion rates and storages for the two zones. This is accomplished for each free water storage as follows:

#### Primary (LZPK and LZFPM)

Select a period when the recession is the flattest (least decay with time) with a minimum of precipitation and calculate a slope during this period.

**Example:**

Primary flow on August 1: 0.42 mm ( $Q_{P_2}$ )

Primary flow on June 1: 0.50 mm ( $Q_{P_1}$ )

Primary daily recession rate ( $K_p$ ) =  $(Q_{P_2}/Q_{P_1})^{1/t}$

where:  $t$  is time in days

$$K_1 = (0.42/0.5)^{1/61} = 0.997$$

$$LZPK = 1 - K = 0.003$$

A value for the maximum free primary water storage may be obtained by dividing the maximum discharge under only primary flow conditions by the daily depletion rate (LZPK). This calculation should be based on the largest value of primary flow which can be observed or estimated from the hydrograph trace.

$$LZFPM = Q_{P_1}/LZPK = 0.42/0.003 = 140 \text{ mm}$$

Supplemental (LZSK and LZFSM)

Computations similar to those used for the primary storage values are used for the supplemental values. In this case, estimates of the primary baseflow contribution to the observed flow must be subtracted before the slope representing the supplemental baseflow is computed.

**Example:**

Period selected: March 1 to April 9

| Discharge              | March 1        | April 9        |
|------------------------|----------------|----------------|
| Observed               | 8.10 mm        | 1.68 mm        |
| Estimated primary      | <u>0.10 mm</u> | <u>0.08 mm</u> |
| Estimated supplemental | 8.00 mm        | 1.60 mm        |

Supplemental daily recession rate ( $K_s$ ) =  $(1.60/8.00)^{1/40} = 0.960$

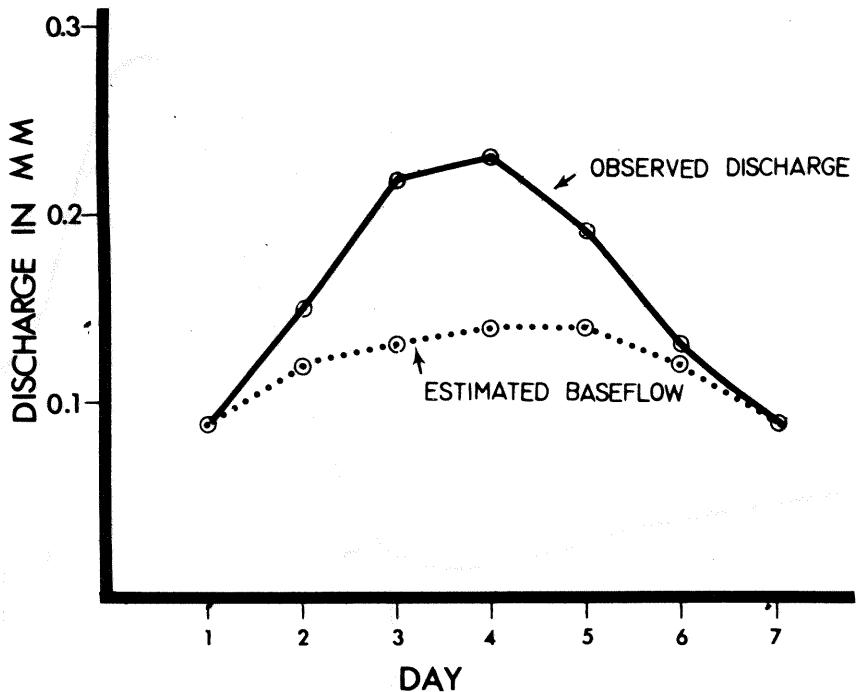
$$LZSK = 1 - K_s = 1 - 0.960 = 0.040$$

and

$$LZFSM = 8.00/0.040 = 200 \text{ mm}$$

Percent Impervious (PCTIM)

A small rise on the hydrograph during an extended dry period may be used to compute a value for PCTIM. This is calculated as shown in figure 3.



| Day | Basin rain (mm) | Observed discharge (mm) | Estimated baseflow (mm) | Estimated direct R.O. (mm) |
|-----|-----------------|-------------------------|-------------------------|----------------------------|
| 1   | 0.0             | 0.09                    | 0.09                    | 0.00                       |
| 2   | 30.0            | .15                     | .12                     | .03                        |
| 3   | 19.0            | .22                     | .13                     | .09                        |
| 4   | 0.0             | .23                     | .14                     | .09                        |
| 5   | 0.0             | .19                     | .14                     | .05                        |
| 6   | 0.0             | .13                     | .12                     | .01                        |
| 7   | 0.0             | 0.09                    | 0.09                    | -.00                       |
|     | 49.0            |                         |                         | 0.27                       |

$$\text{PCTIM estimate} = \frac{\sum \text{Direct R.O.}}{\sum \text{Rain}} = \frac{0.27}{49} = 0.0055$$

Figure 3.-- Calculation of PCTIM

#### Lower Zone Tension Water Maximum (LZTWM)

Select a period following an extended dry period, as indicated on figure 4, where the discharges  $Q_1$  and  $Q_2$  represent only baseflow. A time  $t_1$  should be selected immediately prior to the occurrence of direct and/or surface runoff and time  $t_2$  immediately following a period of interflow.

Upper Zone Free Water Maximum (UZFWM) and Drainage Rate (UZK)

The UZFWM cannot be obtained directly from the interflow recession as can be done for the lower zone storages since it does not produce a straight line on semi-log hydrographs. The upper zone free water storage must satisfy percolation and evaporation demand requirements before any water is discharged to the channel. Thus, it is not a simple depletion as for the lower zone free water storages.

Although UZK cannot be obtained directly from analysis of the hydrograph, it is roughly related to the amount of time that interflow occurs following a period with major direct and surface runoff. The longer the period of interflow, the smaller the value of UZK. If we assume that interflow becomes insignificant when its contribution reduces to about 10% of what it is at maximum rate, then the following simple relation can be used to compute a value for UZK:

$$(1 - UZK)^N = 0.10$$

where: N is the average number of days that interflow is observed.

A value of UZFWM can be determined using the UZK computed above and the discharge, corrected for supplemental and primary baseflow, at the time of the highest interflow with ut surface water contribution. It must be recognized that this is a rather rough estimate. The general range for UZFWM has been found to be from 6 to 85 mm with an average of about 25 mm.

Percolation Water Percentage (PFREE)

An estimate of the relative importance of PFREE can be determined from investigating storms following long dry spells that do produce runoff (UZTW completely filled). If the hydrograph returns to approximately the same baseflow as before (indicating little or no addition to the lower zone free water storages), then PFREE is of little significance and has a very small value ranging from 0 to 0.2. If there is a significant increase in baseflow following this type of storm, then PFREE can have a value as high as 0.5. The nominal value for PFREE is 0.3.

Sub-surface Outflow Along Stream Channel (SSOUT)

It is recommended that the value of zero be used. A value for SSOUT other than zero can be applied only if the Q log plot requires a constant

addition to the baseflow in order to achieve a valid recession characteristic.

#### Fraction of Basin Covered by Streams, Etc. (SARVA)

This factor is determined directly from maps showing water and riparian vegetation areas. SARVA can also be inferred from changes in baseflow associated with changes in ET.

#### Percolation Parameters (ZPERC and REXP)

An understanding of the important role played by the percolation parameters is essential to understanding the model and gaining an ability to properly fit the model. Figure 5 demonstrates the part played by the parameters in determining the maximum rate of percolation in relation to the lower zone soil moisture deficiency (DEFR). This curve represents the rate if the upper zone free water is full.

If the lower zone free water storages are full (and the upper zone free water is also at its maximum), then the rate of percolation is equal to PBASE, which is defined by:

$$PBASE = (LZFPM * LZPK + LZFSM * LZSK)$$

This is the maximum outflow that can occur from the lower zones and under steady conditions would represent the percolation to replace the amount removed from the lower zone free water storages as baseflow. As the lower zone soil moisture becomes deficient, the percolation rate increases. When the lower free water storages are completely dry (100% deficient), the percolation rate (assuming UZFW full) occurs at its maximum rate. This is equal to:

$$\text{Maximum percolation rate} = (1 + ZPERC) * PBASE$$

The shape of the percolation curve is determined by the parameter REXP as shown in figure 5.

Initial values of ZPERC must be estimated using as a guideline some evaluation of the possible maximum percolation rate that would be expected for the basin when the upper zone free water storage is full. The ability to estimate this value would increase as additional basins in an area are fitted. With no other means of estimating REXP, a nominal starting value of 1.80 is suggested.

Once an initial simulation is made, the four parameters controlling the

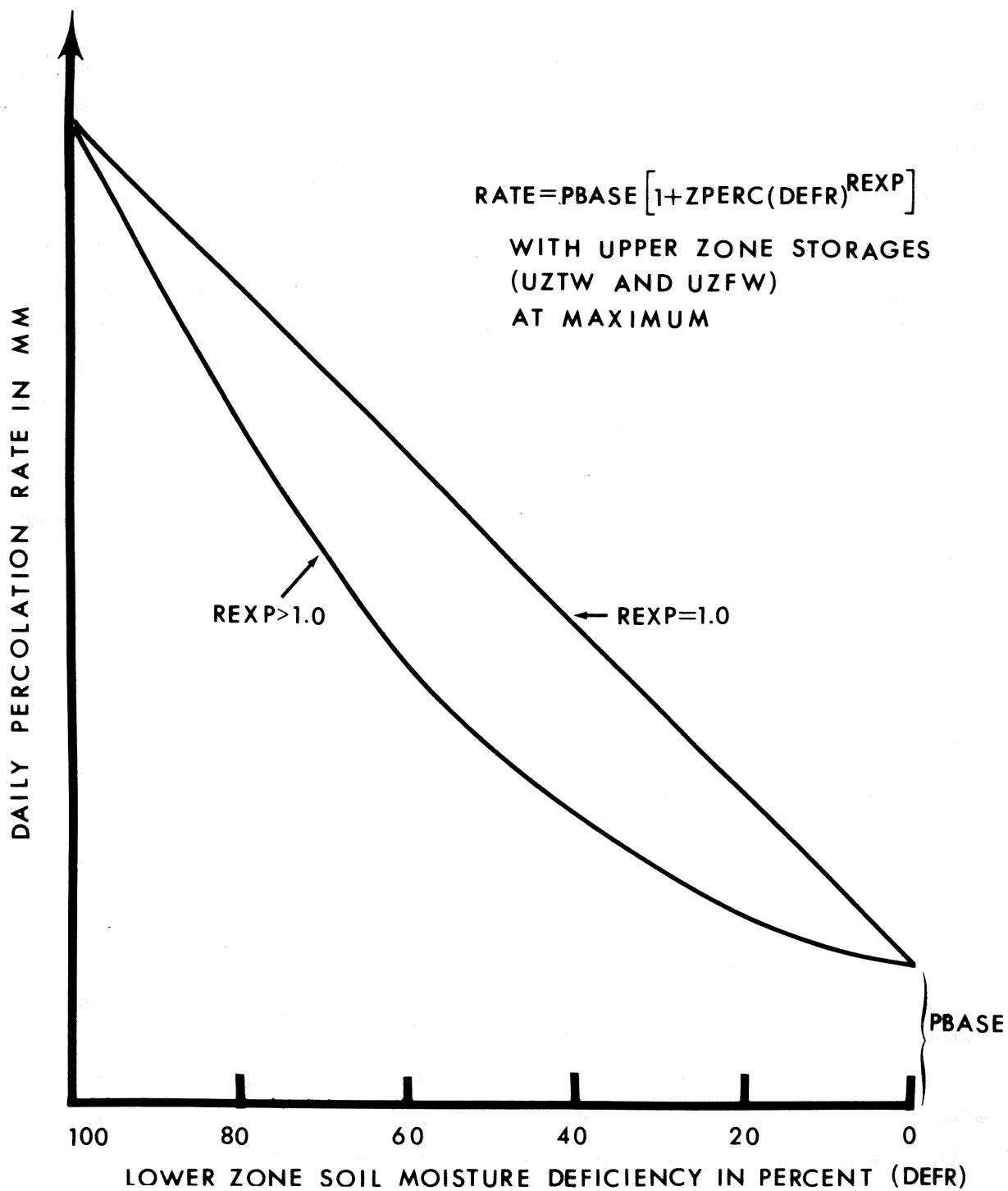


Figure 5.--Percolation representation.

percolation curve are very important for improving the simulation fit. For example, if following an extended dry period the simulated runoff is much less than observed, the percolation curve may be too high for large deficiencies in lower zone storages. Similar analyses of simulated versus observed runoff for periods when the lower zone moisture deficiency would be small will indicate if the curve should be raised or lowered for these conditions. The raising and lowering of the curve can be accomplished by changing ZPERC and/or the value of PBASE. PBASE is related to the maximum values for the lower zone free water storages (LZFSM and LZFPM). The relative values of the supplemental and primary storages are important for the division of the free water contribution to the recession. However, the total value of the storages is primarily important in positioning the percolation curve and may be changed for this purpose. Thus, you should not change the value of ZPERC without considering the necessity to also alter the total capacities of the lower zone free water. The value of REXP allows flexibility in the change in slope over the different values of the lower zone soil moisture deficiency. The fitting of the percolation curve to insure proper initiation of runoff under various lower soil moisture conditions is generally the most important fitting requirement after the first simulation if the volume of runoff is reasonable.

#### Parameters Requiring Nominal Starting Values (SIDE, ADIMP, and RSERV)

Initial value for SIDE is zero. Where it is known from geological or hydrological studies that considerable groundwater bypassed the surface channel, a value other than zero should be used.

The initial value for RSERV is 0.30 and this parameter is generally not optimized.

The additional area of the basin which becomes impervious as all tension water requirements are met (ADIMP) is generally given a nominal starting value of 0.01. Recent investigations suggest that remote sensing techniques using radiation measurements (infrared) can define areas as are indicated by ADIMP. Such measurements may be a means of providing future input for this parameter.

#### SIMULATION FOR SOUTH YAMHILL RIVER

Copies of the worksheets for determination of initial parameters for the South Yamhill River near Whiteson, Oregon, are shown in appendix B. These

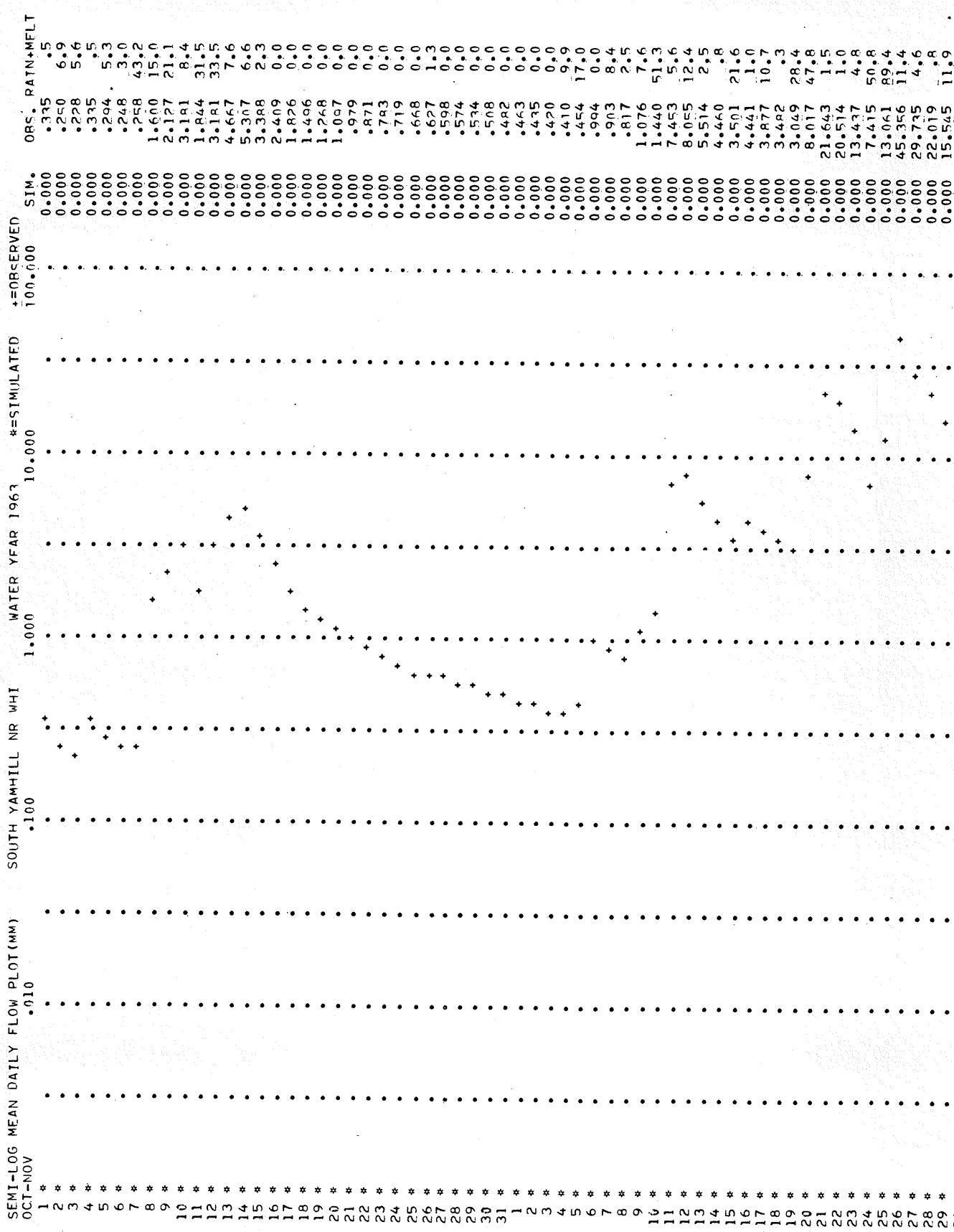
may be used to compare with those obtained in the workshop.

Appendix C contains the copies of the following printouts of the initial simulation.

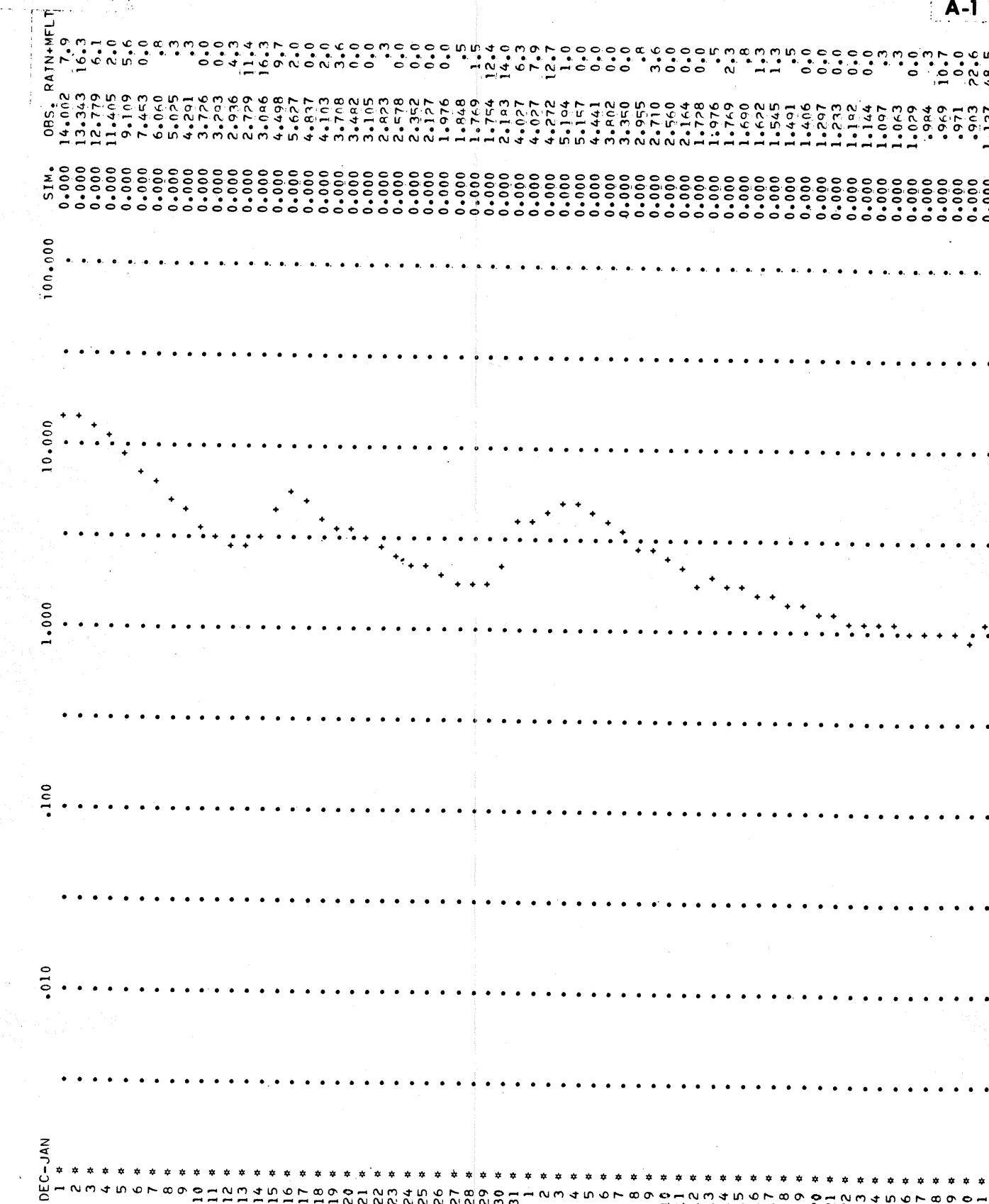
1. Input parameters and other initializing entries.
2. Summation sheet of the statistical summary for the 5-year simulation (Oct. 1962-Sept. 1967).
3. Sample of yearly summary showing soil moisture accounting volumes for each month and listing of soil moisture variables at the end of each month.
4. Semilogarithmic hydrographs for all 5 years of observed and simulated discharges with daily numerical values of the observed discharges, simulated discharges, and liquid water reaching the soil mantle from a combination of rainfall and/or snowmelt (rain + melt).

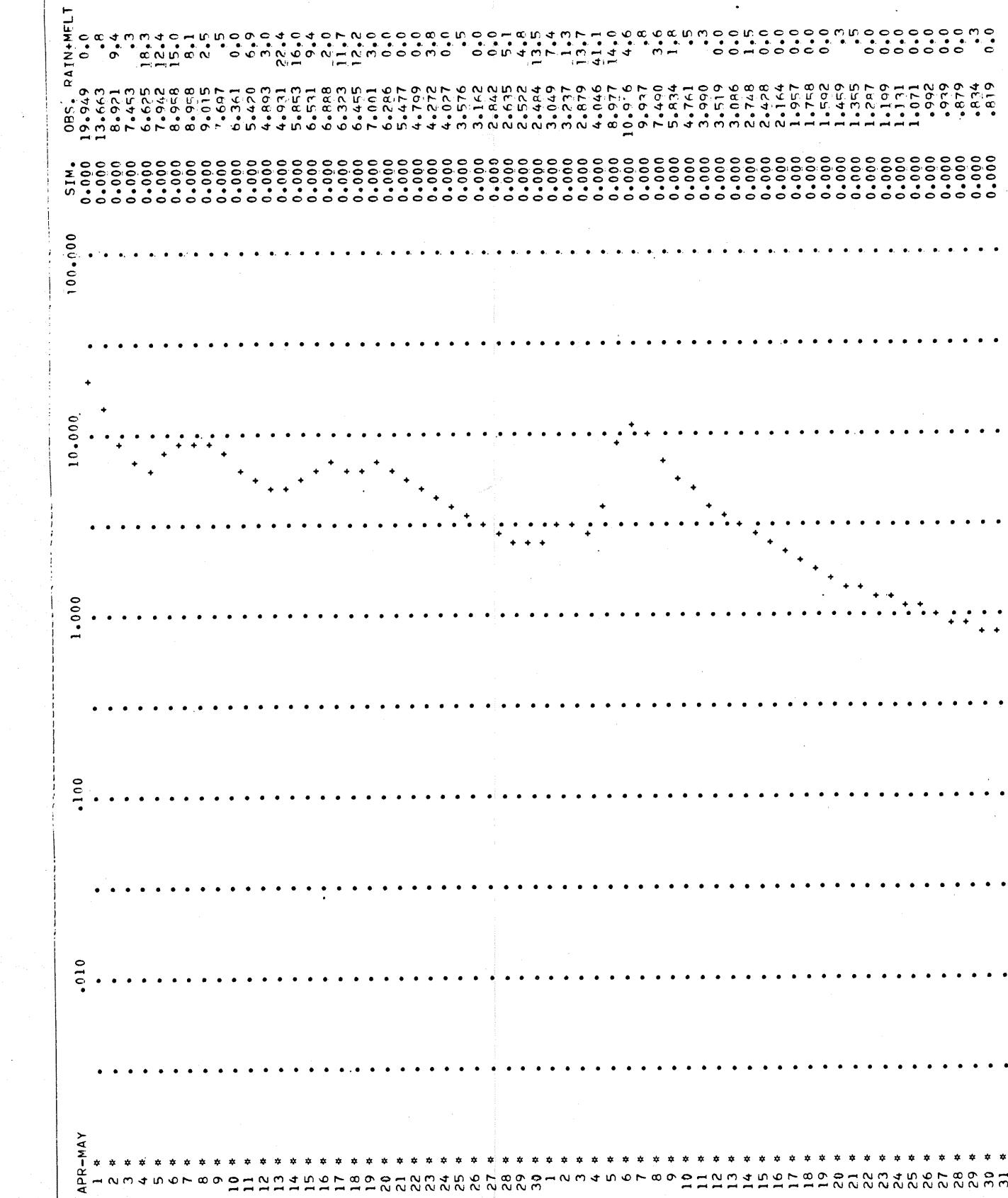
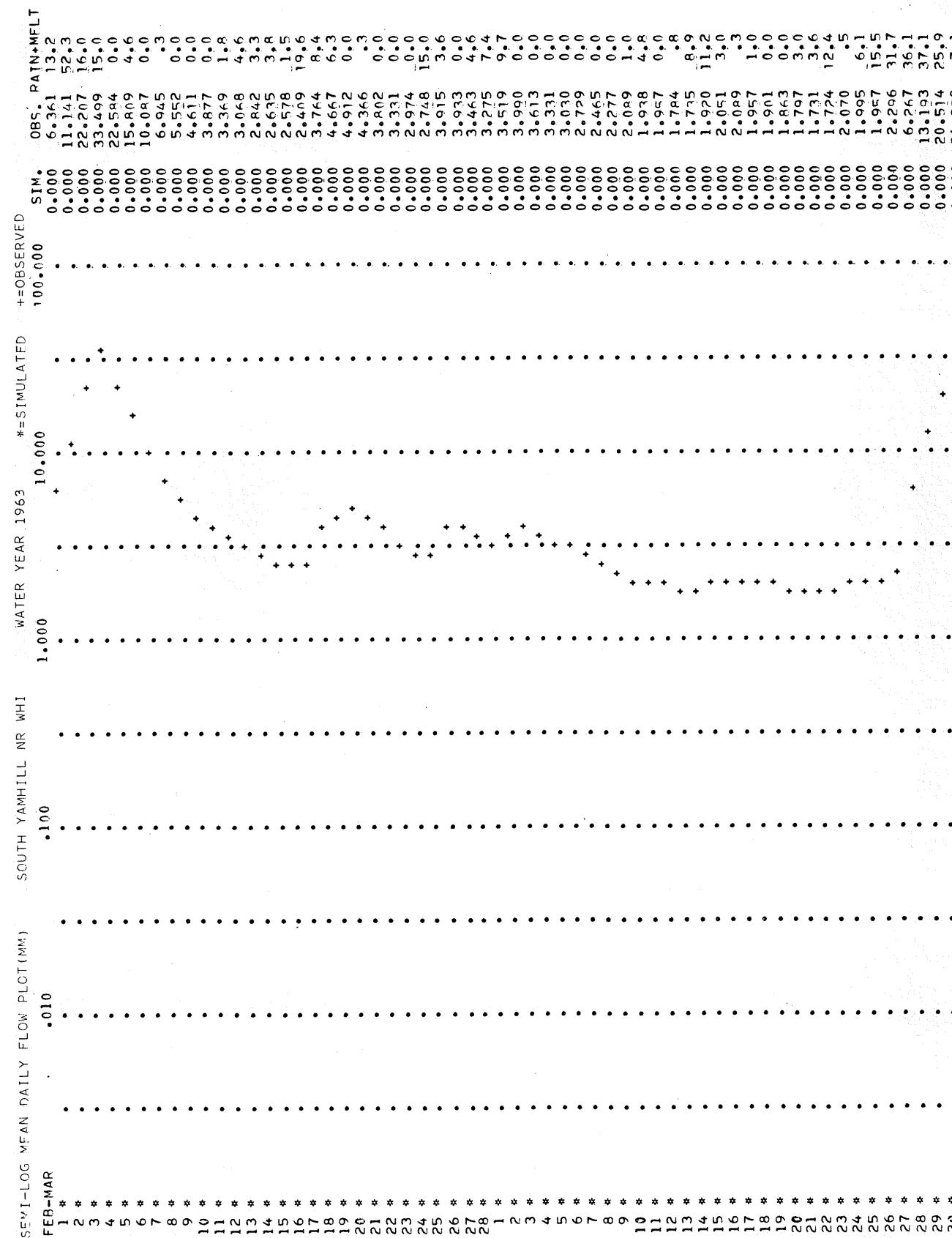
#### ACKNOWLEDGMENTS

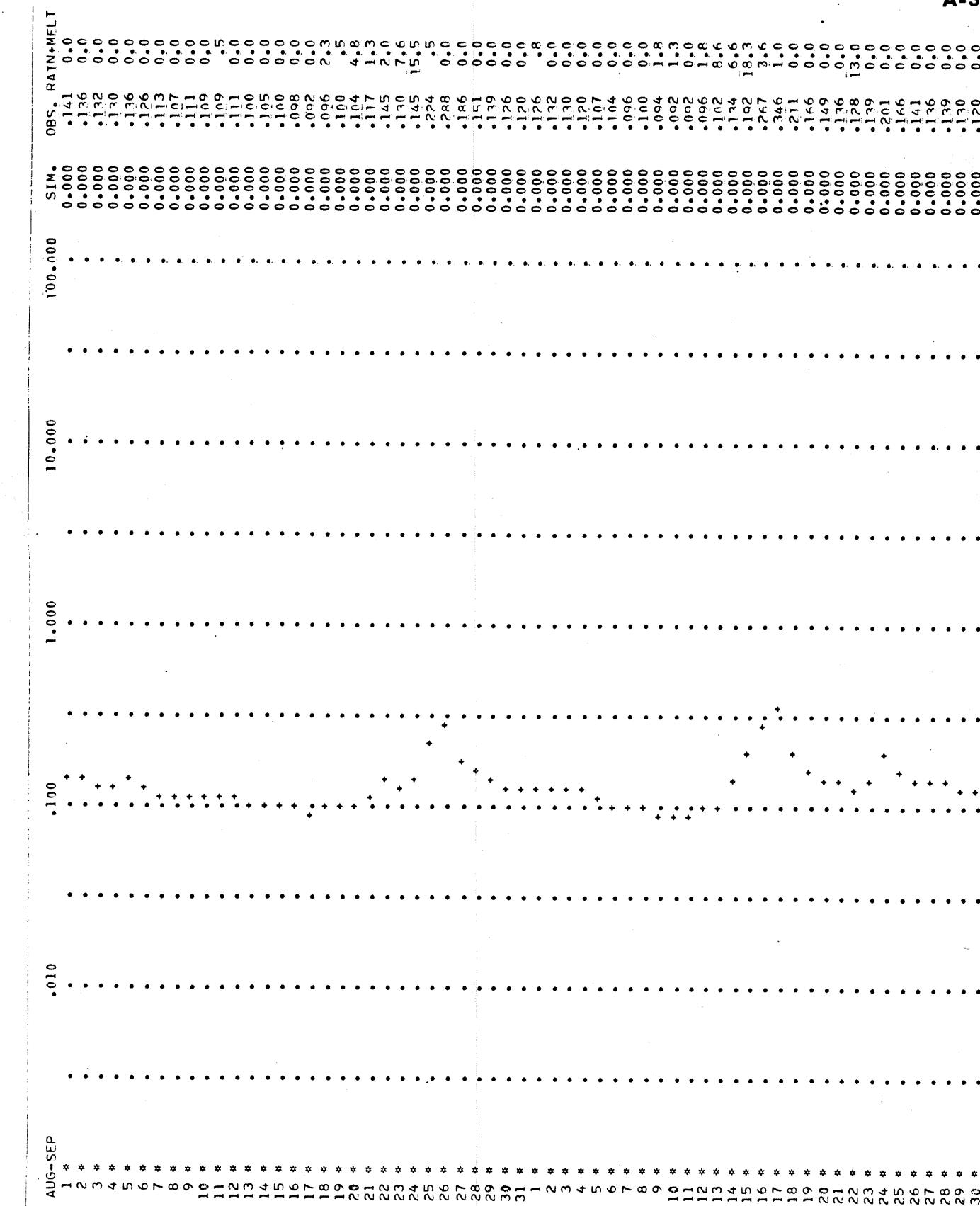
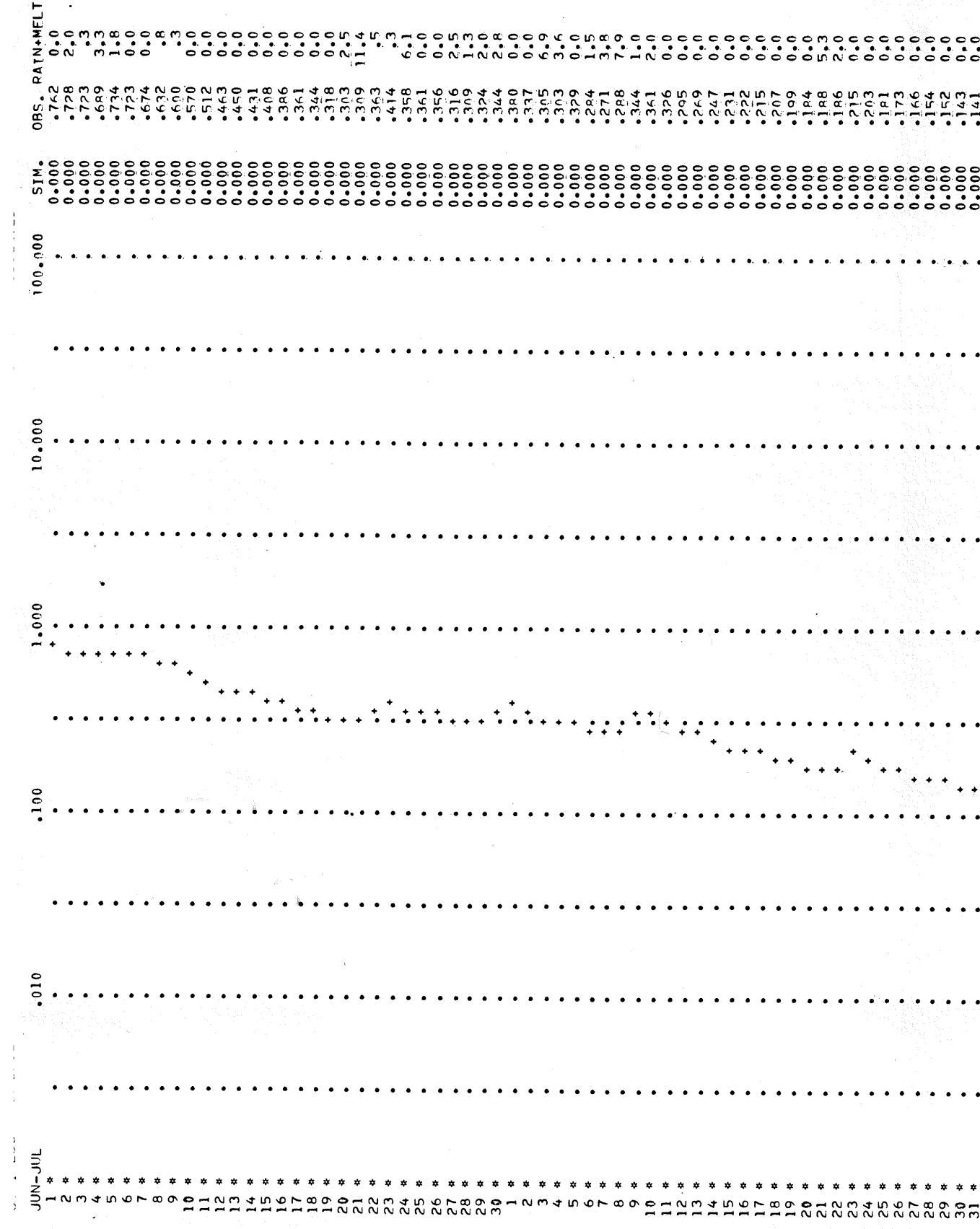
Special thanks are extended to Eric Anderson and Robert Burnash for the technical advice they provided and to the many members of the Hydrologic Research Laboratory who assisted in preparing the material.

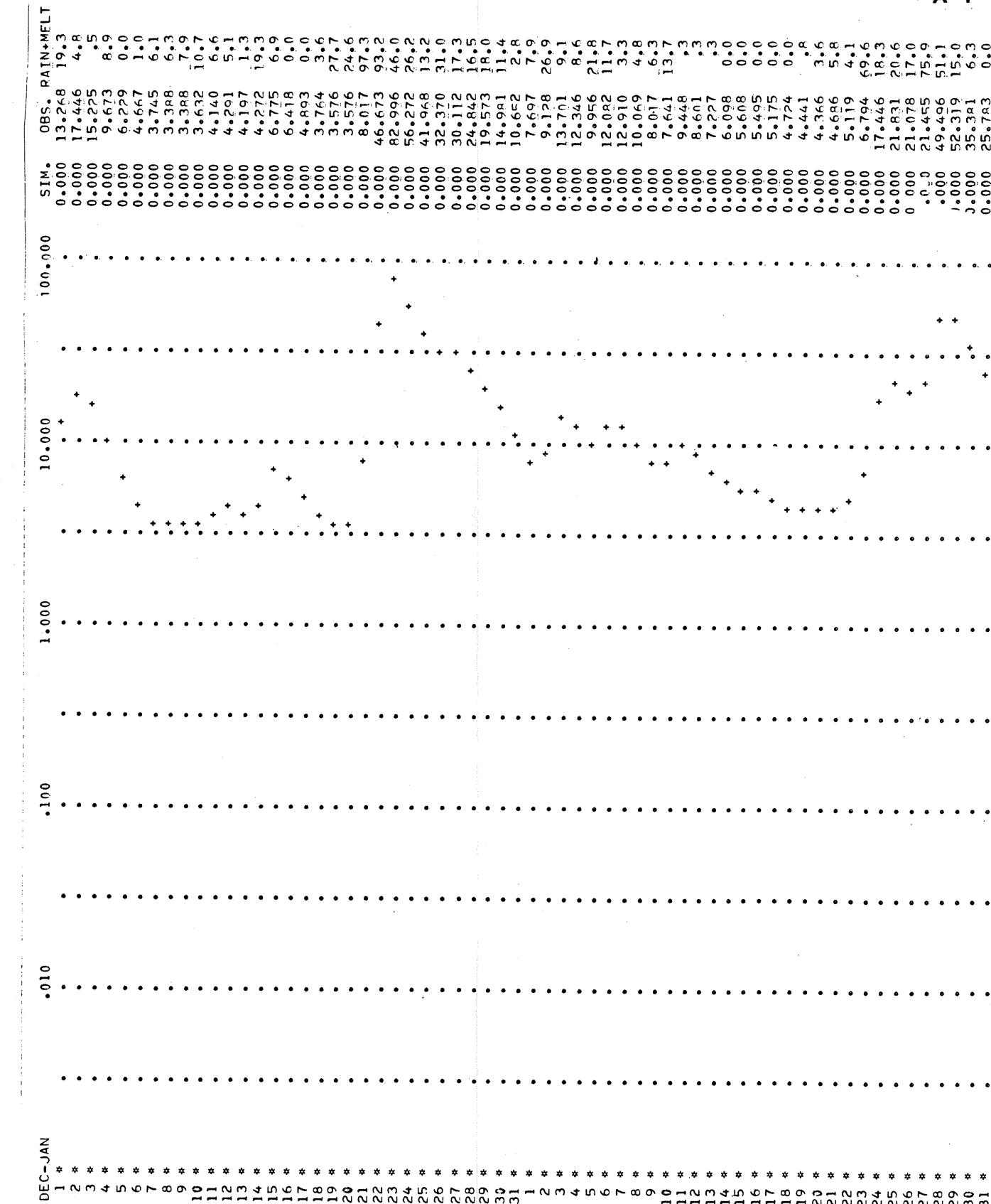
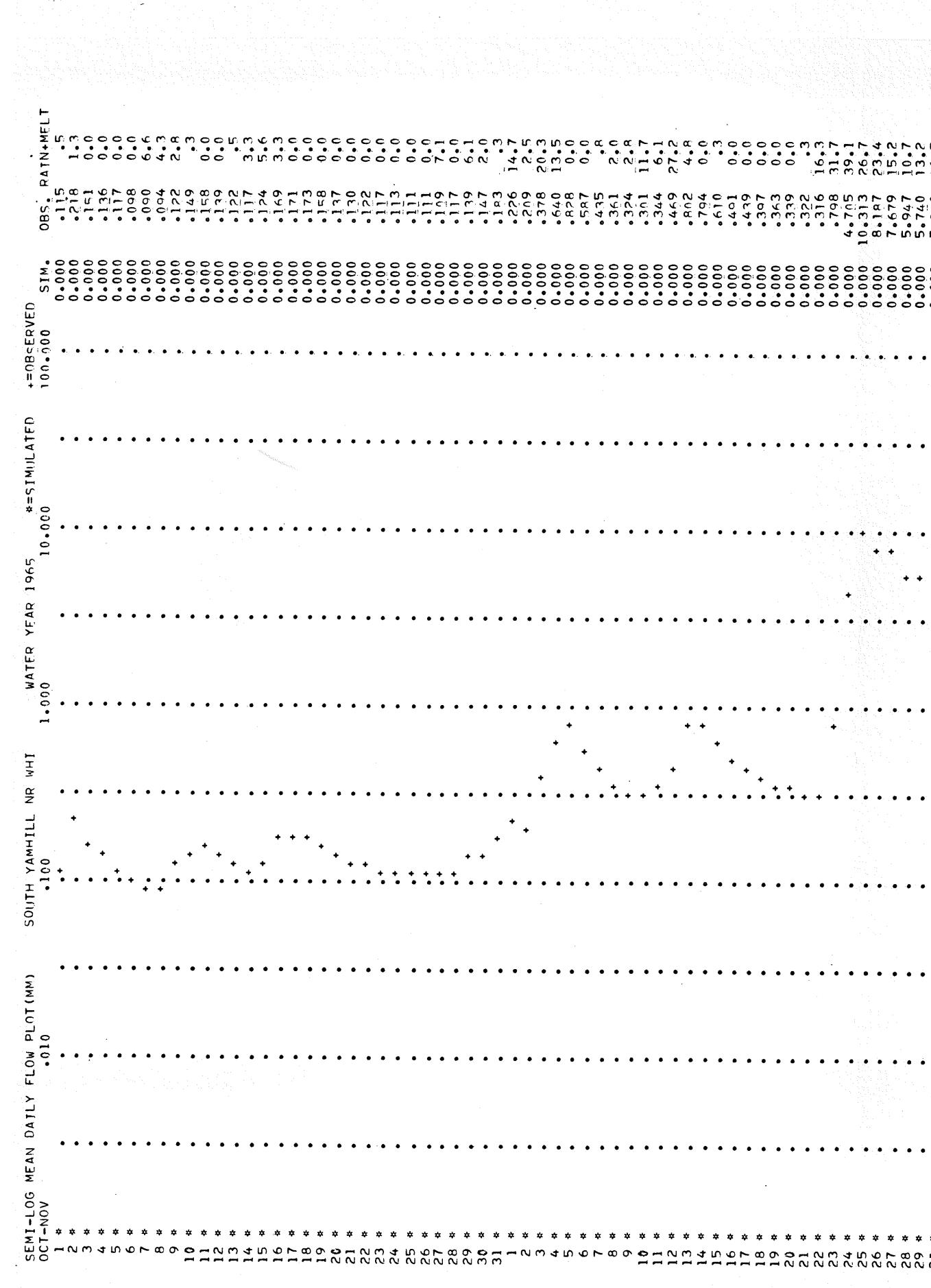


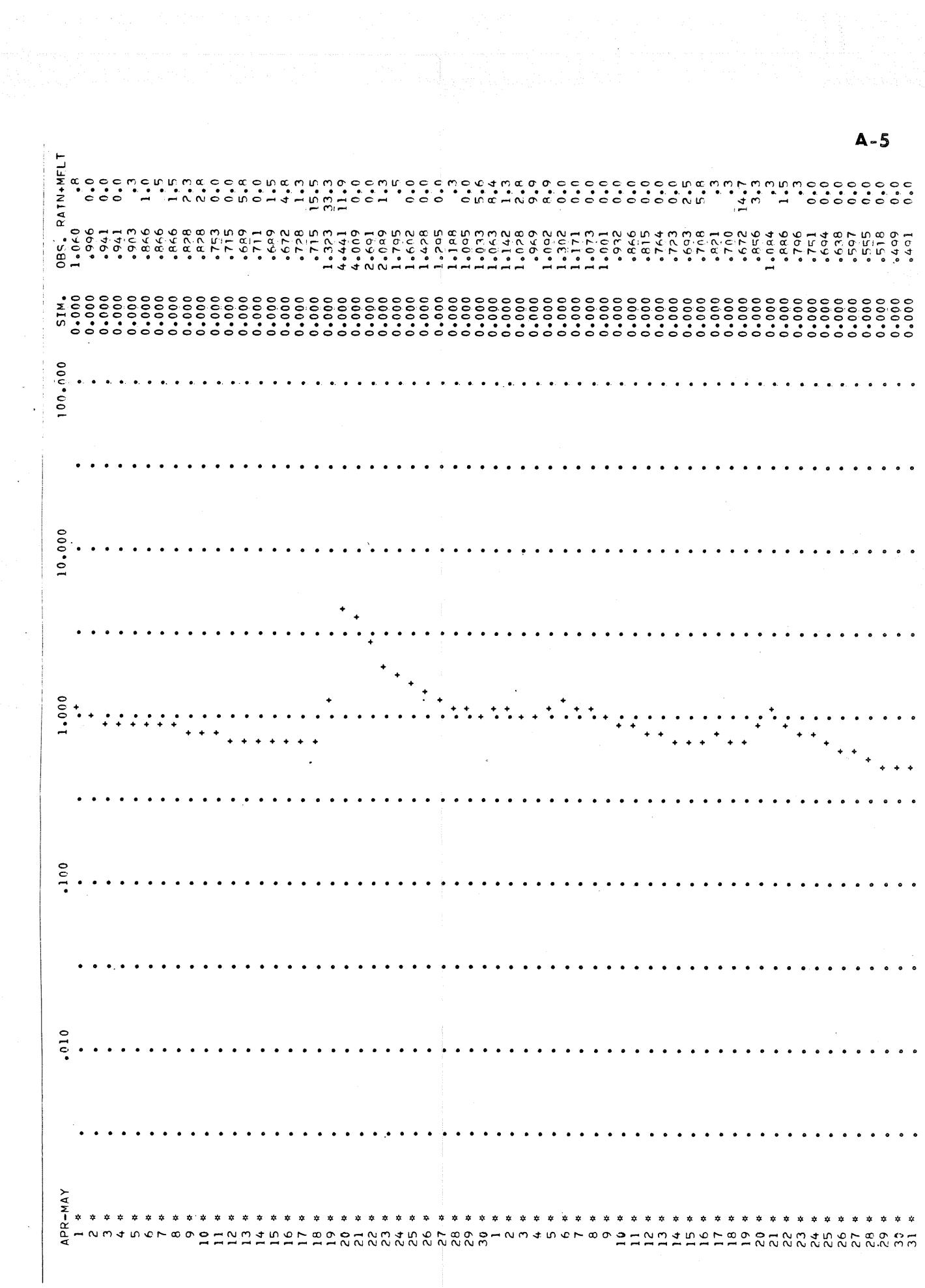
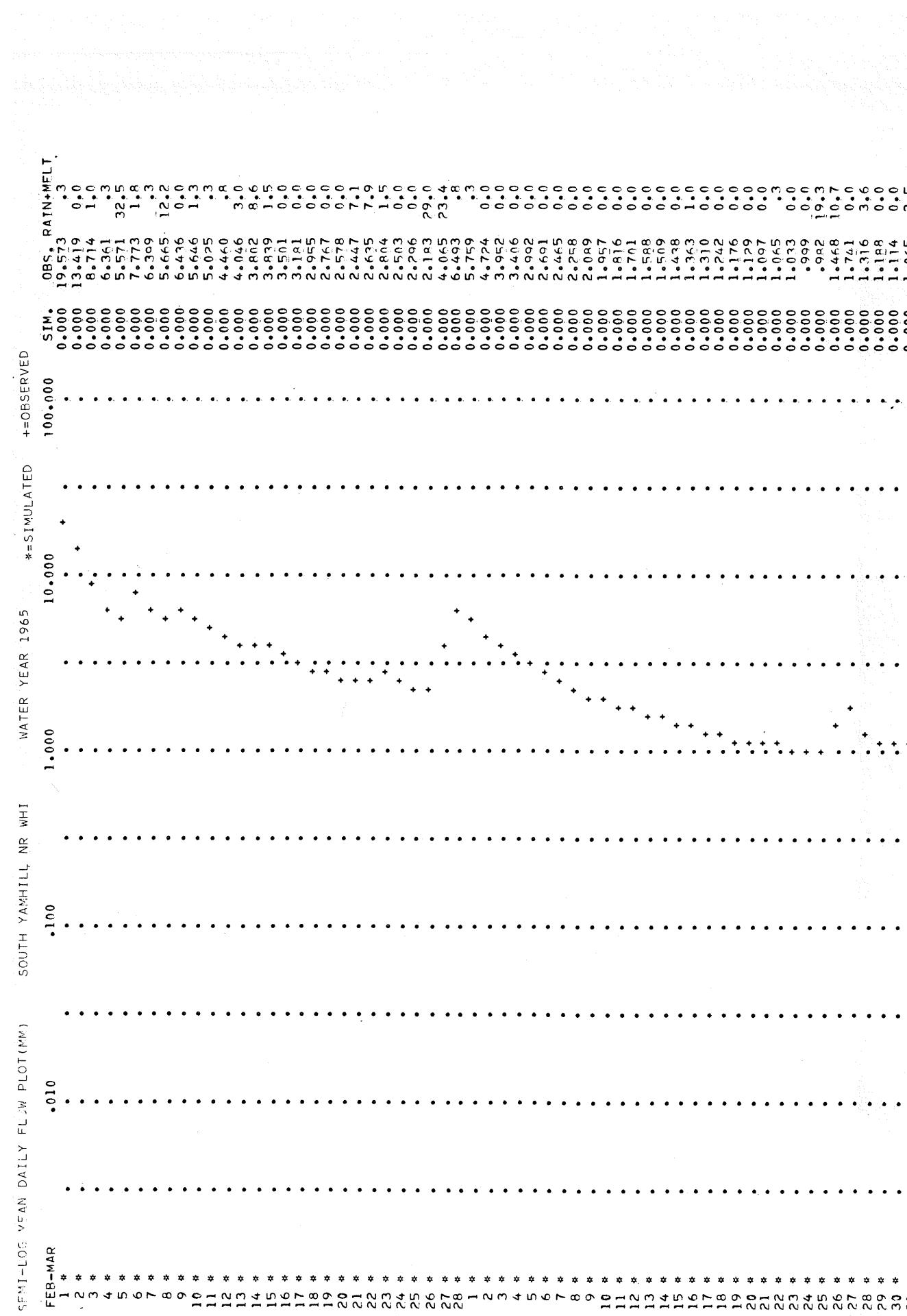
## APPENDIX A

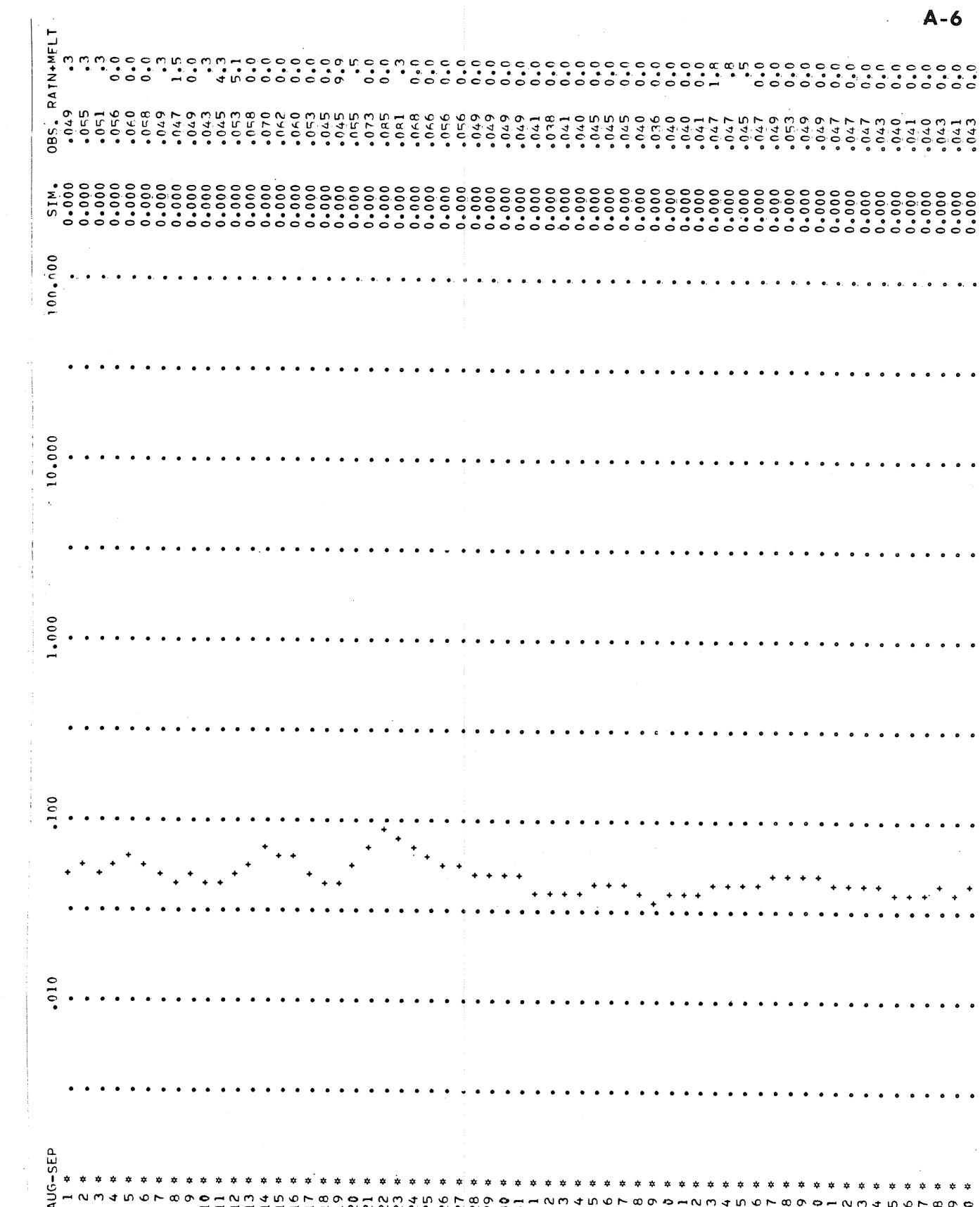
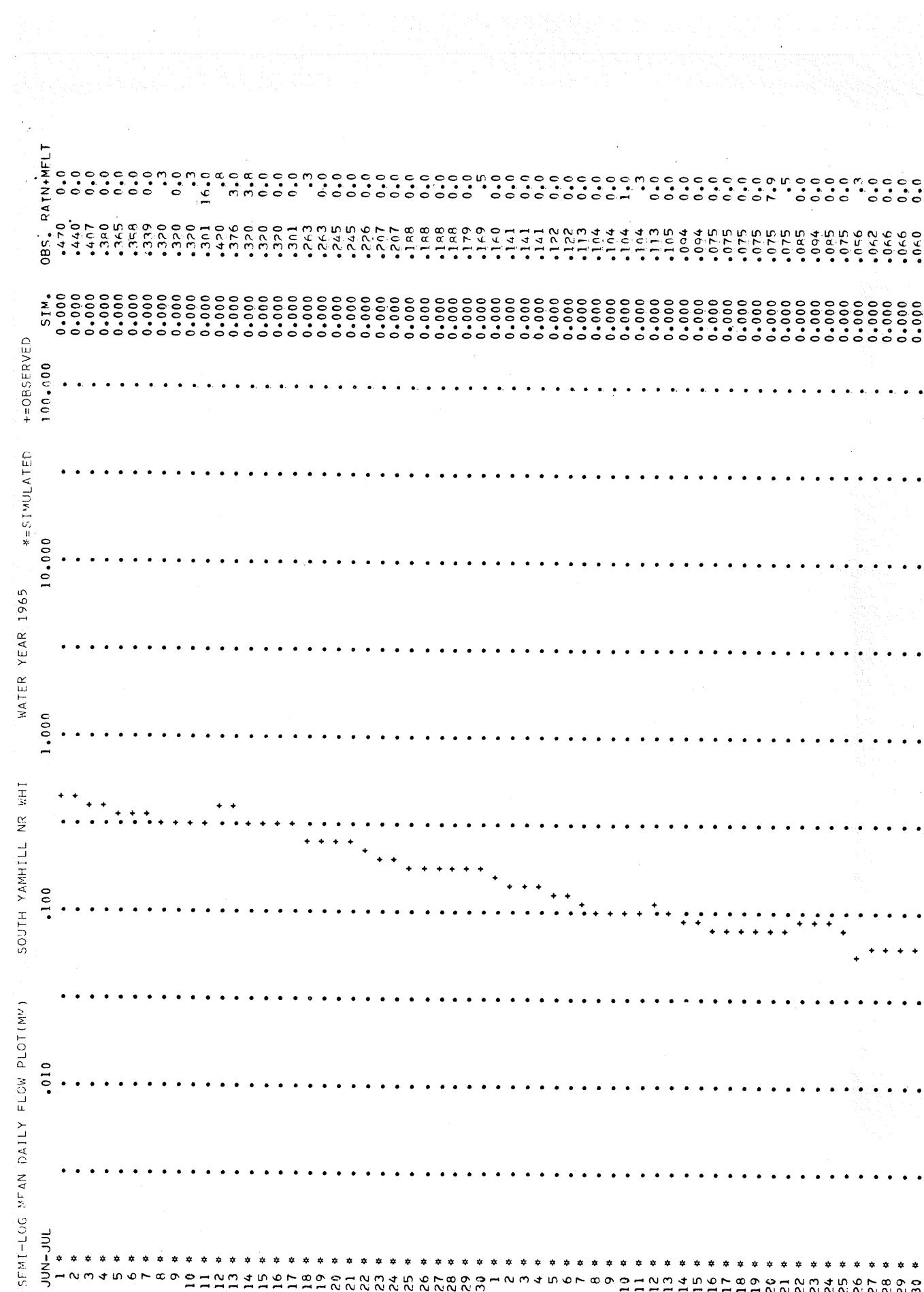








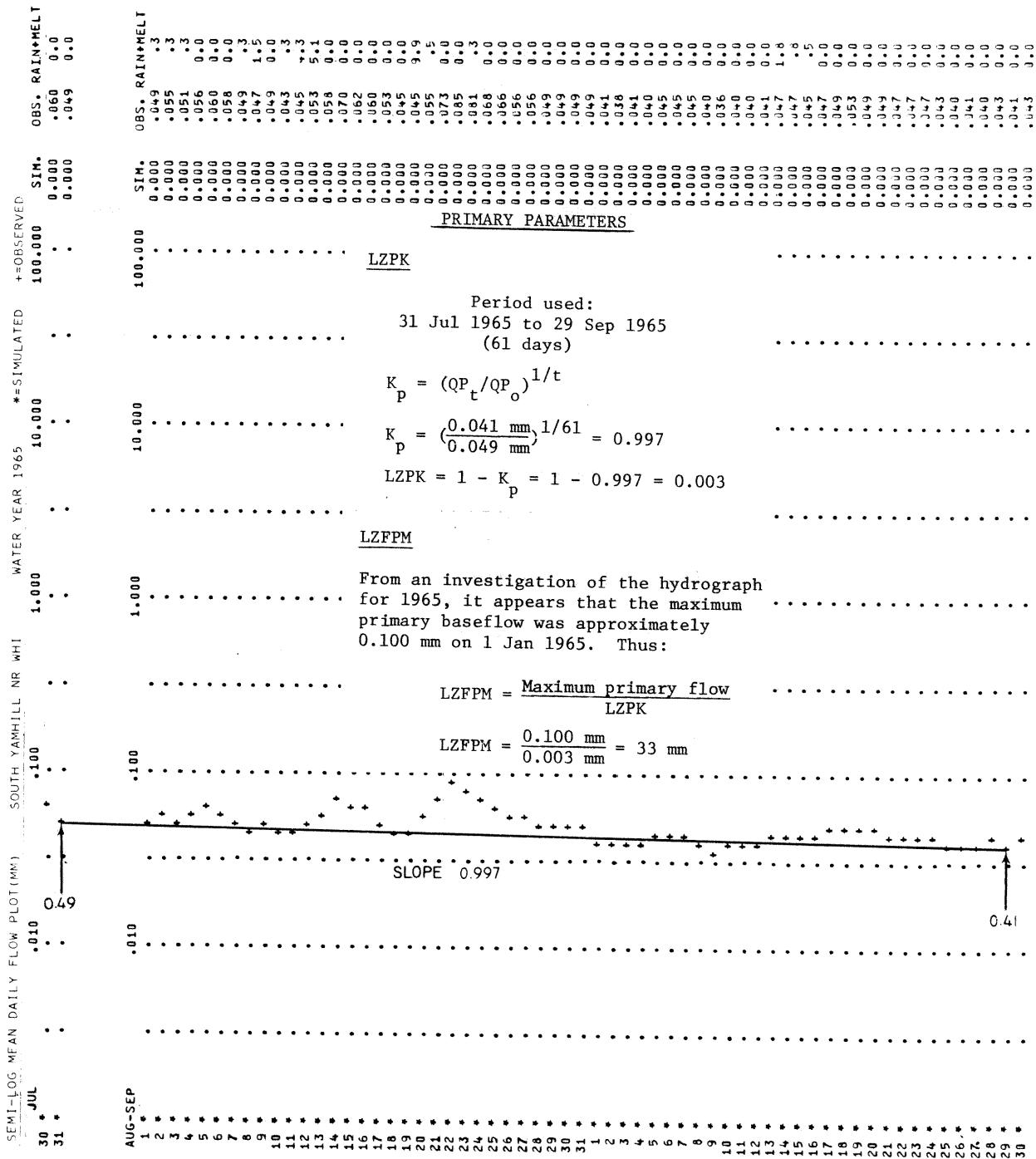


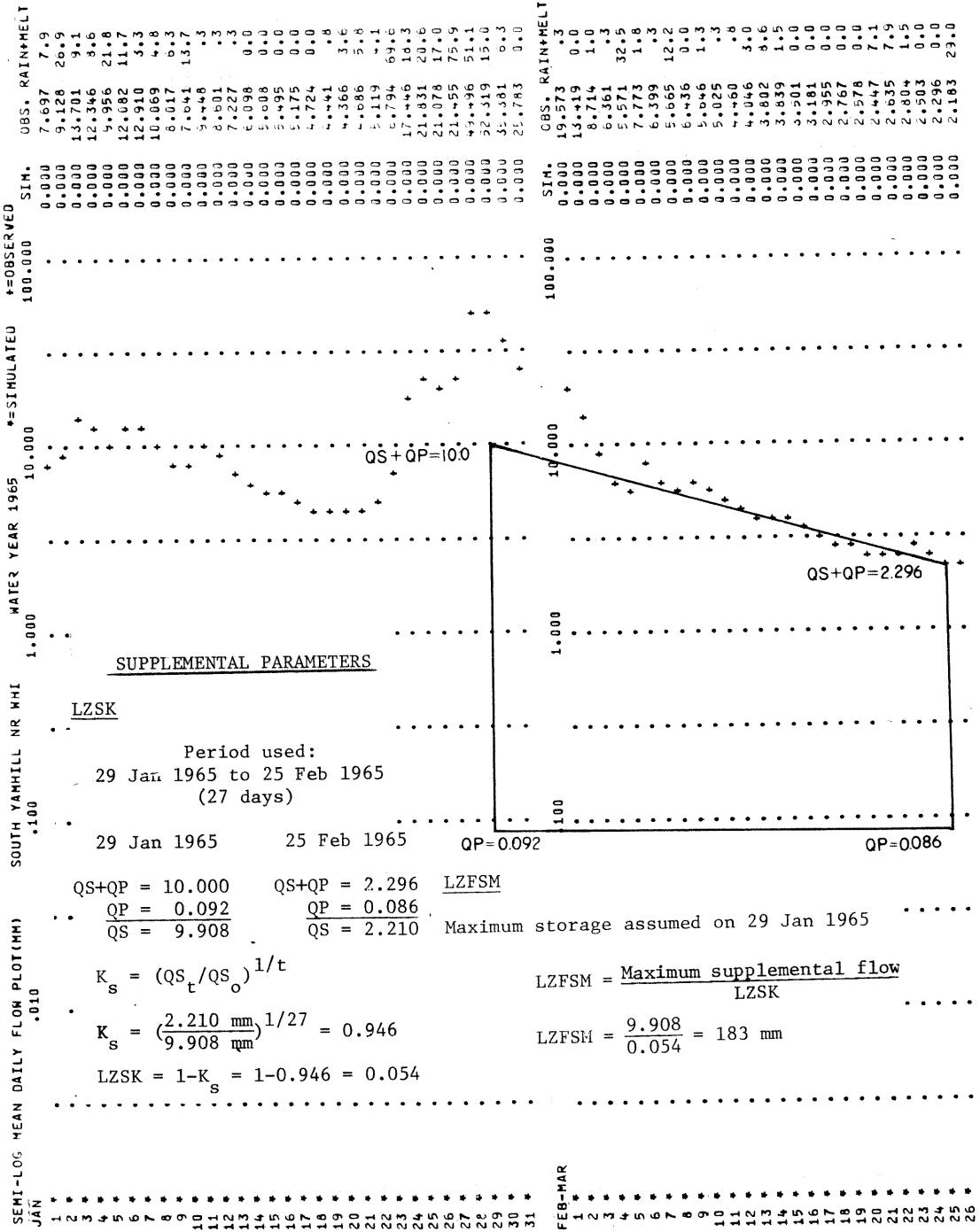


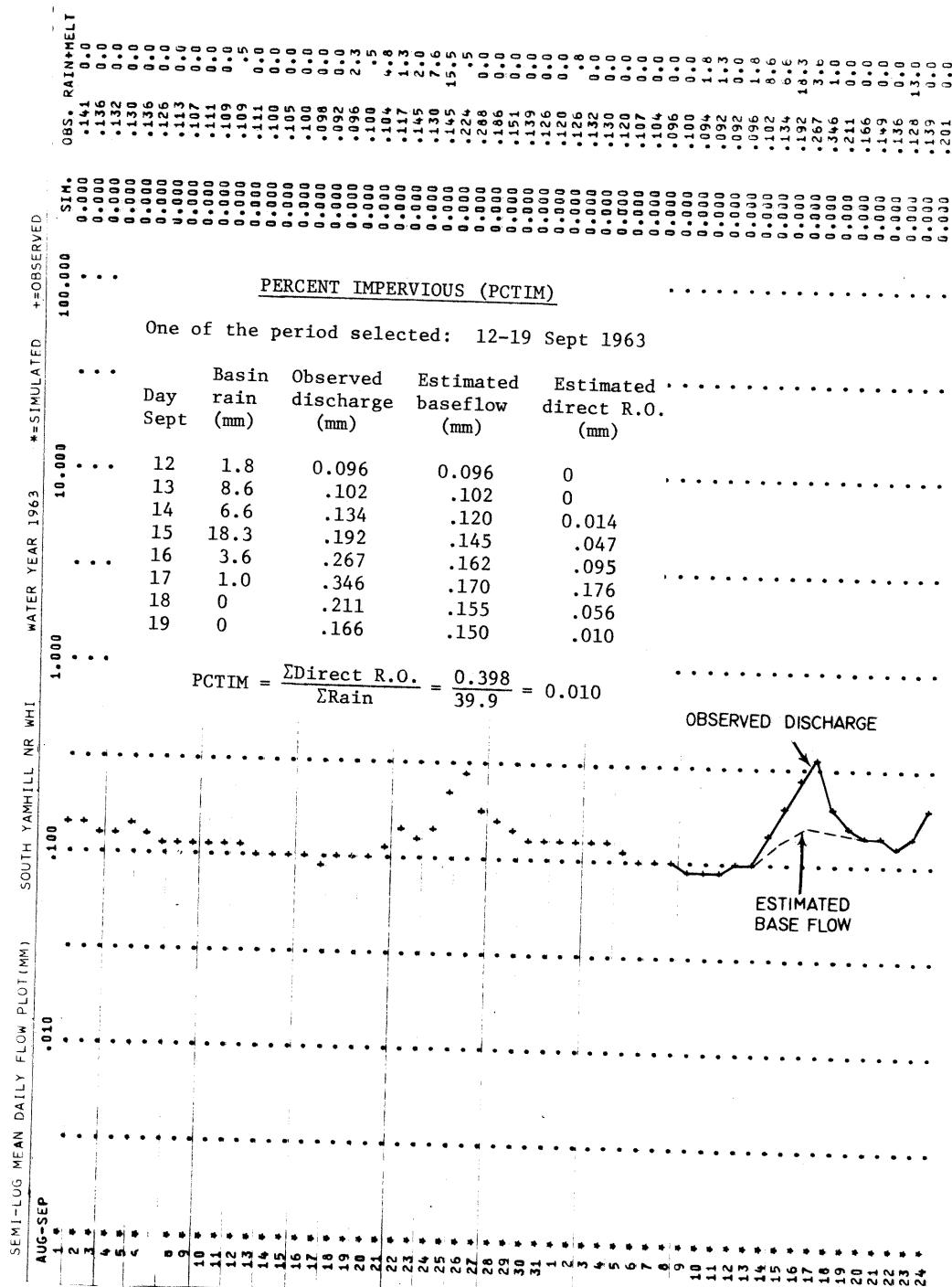
## APPENDIX B

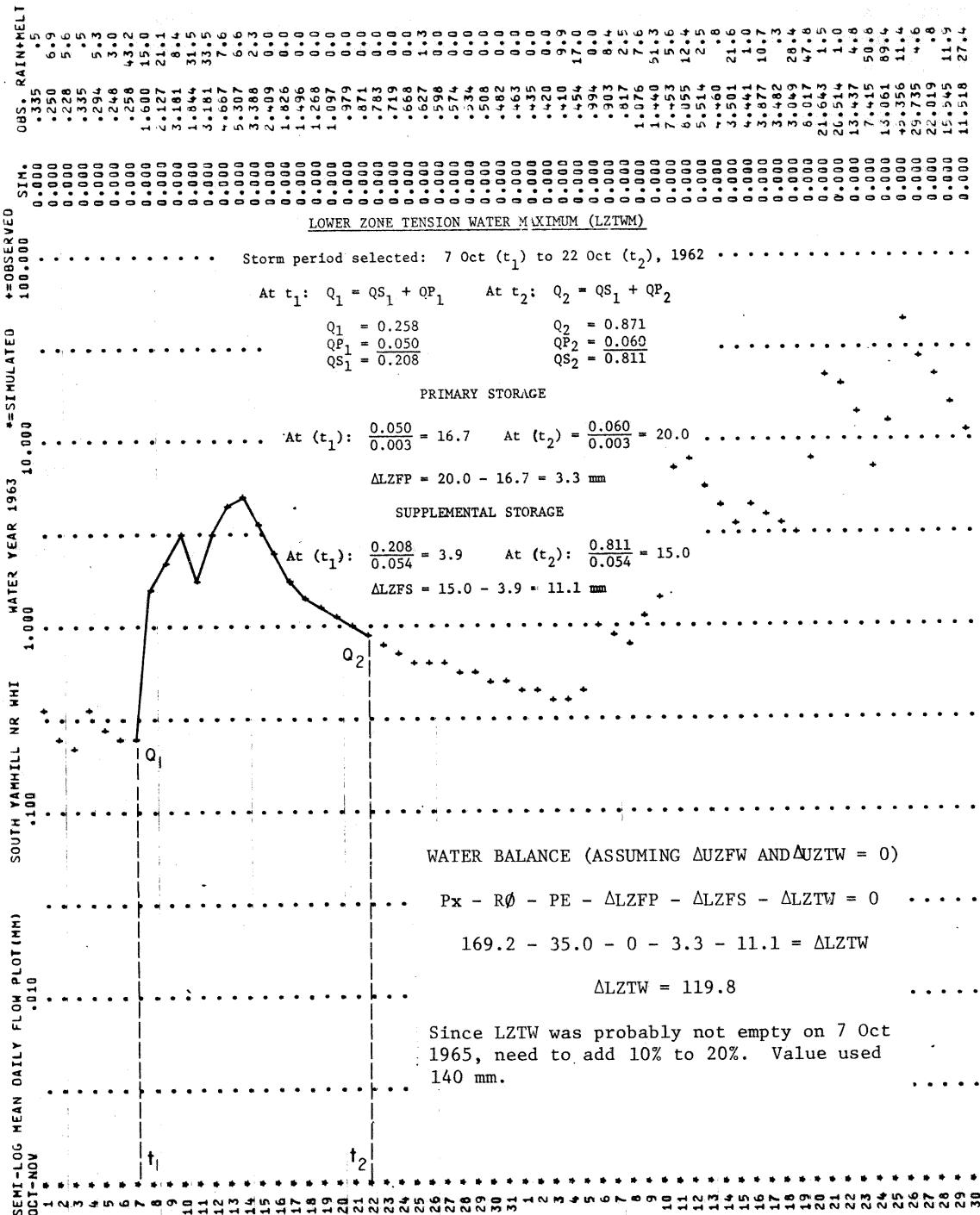
INITIAL VALUES OF SOIL MOISTURE PARAMETERS  
FOR  
SOUTH YAMHILL RIVER NEAR WHITESON, OREGON, U.S.A.

| <u>Page</u> | <u>Parameters</u>   |  | <u>Initial values</u>   |
|-------------|---|--|---|
| B-2         | Primary Parameters  | LZPK<br>LZFPM  | 0.003<br>33 mm  |
| B-3         | Supplemental Parameters   | LZSK<br>LZFSM  | 0.054<br>180 mm   |
| B-4         | Percent Impervious  | PCTIM  | 0.01  |
| B-5         | Lower Zone Tension Water Maximum  | LZTWM  | 140 mm  |
| B-6         | Upper Zone Tension Water Maximum<br>Upper Zone Free Water Drainage Rate<br>Upper Zone Free Water Maximum<br>Percentage Division of Percolation<br>Percentage of Basin Covered by<br>Streams, Etc.<br>Loss Along Stream Channel<br>Parameters with Nominal<br>Initial Values | UZTWM<br>UZK<br>UZFWM<br>PFREE<br>SARVA<br>SSOUT<br>SIDE<br>ADIMP<br>RSERV | 35 mm<br>0.3<br>25 mm<br>0.3<br>0.01<br>0.00<br>0.0<br>0.01<br>0.30 |
| B-7         | Percolation Parameters  | ZPERC<br>REXP  | 8<br>1.80   |
| B-8         | Percolation Representation  |  |   |









UPPER ZONE TENSION WATER MAXIMUM (UZTWM)

From a review of small storms following dry periods (data listed in text of report), a value of 35 mm was selected as representing the lower limit of the maximum amount required by upper zone tension water before overflow occurs, from the upper zone.

UPPER ZONE FREE WATER DRAINAGE RATE (UZK)

Interflow for the South Yamhill appears to last about 7 days.

Using N = 7 in the following equation:

$$(1 - UZK)^N = 0.10$$

an approximate value of 0.3 is obtained for UZK.

UPPER ZONE FREE WATER MAXIMUM (UZFWM)

The unit hydrograph for the basin indicates a fair delay in water actually reaching the channel. Thus, a considerable portion of the water during the interflow period originally developed as direct and surface runoff. In reviewing the storm period of 22 Jan to 3 Feb 1965, the flow on 31 Jan was about 25 mm, of which about 10 mm would be baseflow and about half of the remainder delayed surface and direct runoff. (See page B-3.) With a UZK value of 0.3, we would obtain a value of about 25 mm for UZFWM (8/0.3).

PERCENTAGE DIVISION OF PERCOLATION (PFREE)

Hydrographs of storms having surface runoff following long dry periods were analyzed. For these conditions, UZFW was completely filled and some water could have been available for percolation. For the South Yamhill, the baseflow after such storms appeared to be much higher than prior to the storm. Therefore, a rather large value (0.5) was assigned for PFREE as compared to the average value of 0.3.

PERCENTAGE OF BASIN COVERED BY STREAMS, ETC. (SARVA)

From maps, this was estimated to be 0.01 of the basin.

LOSS ALONG STREAM CHANNEL (SSOUT)

No evidence of loss from baseflow hydrograph. Use zero.

SIDE, ADIMP, and RSERV

Nominal starting values were used for these parameters:

$$(SIDE = 0.0; ADIMP = 0.01; and RSERV = 0.30)$$

PERCOLATION PARAMETERS (ZPERC and REXP)

A daily maximum percolation rate curve (with upper zone storages UZTW and UZFW at maximum) was developed for the basin (fig. B-1). PBASE was computed as 9.819 mm from the equation:

$$\text{PBASE} = \text{LZFPM} * \text{LZPK} + \text{LZFSM} * \text{LZSK}$$

Calculations of parameters used in this equation were developed on pages B-2 and B-3. Based on experience with other basins, a value of approximately 90 was selected for the maximum percolation rate (for the conditions as stated).

For completely dry lower zone conditions (lower zone 100% deficient), the maximum rate is defined as:

$$\text{Maximum rate} = (1 + \text{ZPERC}) * \text{PBASE}$$

$$90 \text{ mm} = (1 + \text{ZPERC}) * 9.819$$

$$\text{ZPERC} = \frac{90 - 9.819}{9.819} = 8.17$$

A value of eight was selected for ZPERC and the nominal initial value of 1.80 was used for REXP.

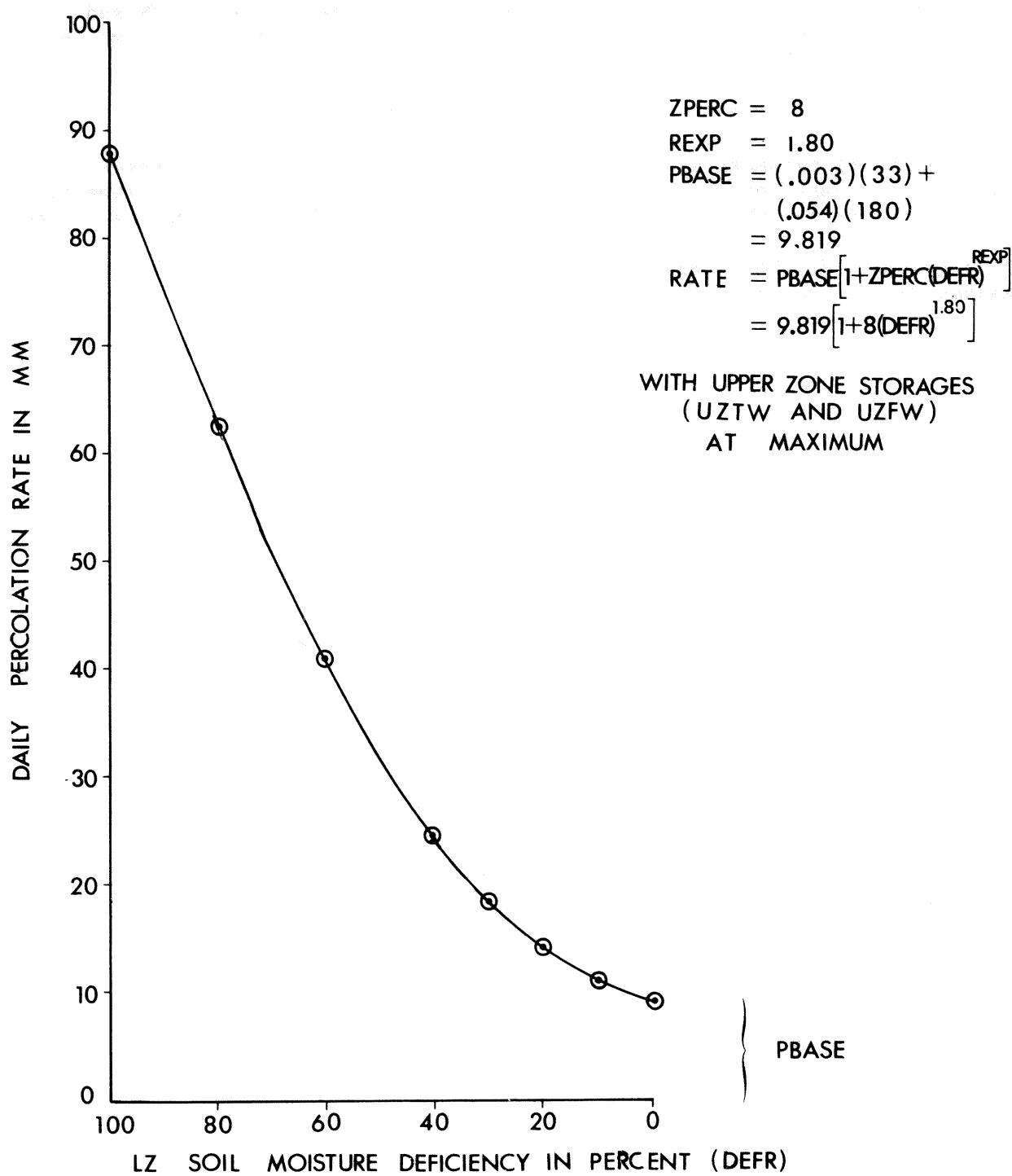


Figure B-1.--Percolation representation for South Yamhill.

## APPENDIX C

SOUTH YAMHILL NEAR WHITESON•OREGON  
 RUN BEGINS OCT 1962      RUN ENDS SEPT 1967

SOIL-MOISTURE ACCOUNTING PARAMETERS      SOUTH YAMHILL NEAR WHITESON•OREGON  
 U.S. NATIONAL WEATHER SERVICE RIVER FORECAST SYSTEM - MANUAL CALIBRATION PROGRAM  
 CONTENT AND CAPACITY VALUES ARE IN MM.

## UPPER ZONE AND IMPERVIOUS AREA PARAMETERS

| AREA NO. | ARFA T.D. | AREA NAME      | PX-ADJ | PE-ADJ | UZTWM | U7FWM | UZK  | PCTIM | ADIMD | CAVVA |
|----------|-----------|----------------|--------|--------|-------|-------|------|-------|-------|-------|
| 1        | 14194000  | YAMHILL NR MRP | 1.000  | 1.000  | 35.   | 25.   | .300 | .010  | .010  | .010  |

## PERCOLATION AND LOWER ZONE PARAMETERS

| AREA NO. | PBASE | ZPERC | REXP | LZTWM | L7FWM | L7FPM | L7SK  | L7PK  | PFREEF | RSERV | SIDE |
|----------|-------|-------|------|-------|-------|-------|-------|-------|--------|-------|------|
| 1        | 9.8   | 8.0   | 1.80 | 140.  | 180.  | 33.   | 0.540 | .0030 | .50    | .30   | 0.00 |

PE-ADJUSTMENT OR FT-DEMAND FOR THE 16TH OF EACH MONTH

| AREA NO. | FT-DEMAND-MM/DAY | 1  | 2   | 3   | 4   | 5   | 6   | 7   | 8   | 9   | 10  | 11  | 12  | I.D. OF PE DATA |
|----------|------------------|----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----------------|
| 1        | .5               | .5 | 1.0 | 1.8 | 2.5 | 2.8 | 3.0 | 3.0 | 2.8 | 2.3 | 2.0 | 1.0 | 1.0 |                 |

## INITIAL STORAGE CONTENTS

| AREA NO. | UZTWC | UZFWC | LZTWC | LZFWC | L7FSC | L7FPC | ADIMC |
|----------|-------|-------|-------|-------|-------|-------|-------|
| 1        | 25.   | 0.    | 25.   | 0.    | 0.    | 0.    | 25.   |

SOUTH YAMHILL NEAR WHITESON•OREGON

U.S. NATIONAL WEATHER SERVICE RIVER FORECAST SYSTEM - MANUAL CALIBRATION PROGRAM

## FLOW-POINT PARAMETERS

| NO. | FLOW-POINT NAME      | ARFA-SQ KM | K    | SQOUT | ORSFR | COMPARE | SIXIN | J | HISTOGRAMS | TIME-DELAY | .009 | .082 | .142 | .181 | .173 | .116 | .081 | .057 | .044 |
|-----|----------------------|------------|------|-------|-------|---------|-------|---|------------|------------|------|------|------|------|------|------|------|------|------|
| 1   | SOUTH YAMHILL NR WHI | 1300.00    | 3.01 | 0.00  | 0     | 1       | 0     | 0 | 0          | 0.011      | .033 | .013 | .023 | .011 | 1    | 1    | 1    | 1    | 1    |

MULTIYEAR STATISTICAL SUMMARY

**FLOWPRINT = SOUTH YAMHILL NR WHI**

| MONTH      | SIMULATED<br>MEAN | OBSERVED<br>MEAN | BIAS<br>(SIM<br>MEAN<br>-OBS<br>MEAN) | 1ST MOMENT |                   |                        | STANDARD<br>ERROR<br>FROP<br>MOMENT(OBS) | PERCENT<br>CORREL.<br>COEFF. | BEST FIT LINE          |       |
|------------|-------------------|------------------|---------------------------------------|------------|-------------------|------------------------|--|------------------------------|------------------------|-------|
|            |                   |                  |                                       | PERCFNT    | (SIM)-1ST<br>BIAS | MAXIMUM<br>MOMENT(OBS) |  |                              | OBS = A + R * SIM<br>A | R     |
| OCTOBER    | 9.186             | 7.642            | 1.544                                 | 20.211     | 1.536             | -14.613                | 3.975                                    | .951                         | .055                   | .826  |
| NOVEMBER   | 56.895            | 61.096           | -4.201                                | -6.876     | -7.94             | -156.641               | 28.853                                   | .941                         | -2.234                 | 1.113 |
| DECEMBER   | 121.384           | 127.250          | -5.866                                | -4.610     | .380              | -211.356               | 45.523                                   | .962                         | -10.201                | 1.132 |
| JANUARY    | 156.310           | 166.336          | -9.506                                | -7.715     | -6.78             | 233.231                | 50.943                                   | .947                         | -6.176                 | 1.100 |
| FEBRUARY   | 87.163            | 78.126           | 9.037                                 | 11.568     | 1.206             | -155.589               | 26.791                                   | .900                         | -46.835                | 1.434 |
| MARCH      | 85.714            | 80.944           | 4.771                                 | 5.894      | -24.9             | -104.97                | 22.398                                   | .961                         | -13.896                | 1.104 |
| APRIL      | 47.436            | 42.682           | 4.754                                 | 11.139     | .590              | -110.329               | 14.407                                   | .929                         | -9.269                 | 1.095 |
| MAY        | 19.681            | 19.187           | •4.94                                 | 2.573      | •1.22             | -24.018                | 6.390                                    | .963                         | -1.011                 | 1.026 |
| JUNE       | 4.306             | 5.094            | -7.88                                 | -15.471    | -1.039            | 7.979                  | 1.232                                    | .792                         | 3.265                  | 4.425 |
| JULY       | 1.320             | 2.069            | -7.49                                 | -36.183    | •0.66             | -2.926                 | .740                                     | .775                         | .622                   | 1.096 |
| AUGUST     | •74.9             | 1.092            | -344                                  | -31.445    | •491              | -2.890                 | .705                                     | .557                         | -241                   | 1.781 |
| SEPTEMBER  | 90.8              | 1.143            | -236                                  | -20.597    | 1.093             | 1.951                  | .539                                     | .684                         | .466                   | .746  |
| WATER YEAR | 49.247            | 49.408           | -160                                  | -325       | 27.924            | 233.231                | 25.18n                                   | .964                         | -4.185                 | 1.088 |

\* \* \*NOTE\*: SUM OF (SIM-OBS) \* \* 2 =

1257419.....ROOT MEAN OF SUM OF (SIM-ORS)\*\*2 = 26.242....\*\*

| FLOW<br>INTERVAL | NUMBER<br>OF CASES<br>OBSERVED | SIMULATED<br>MEAN | BIAS    | PERCENT<br>RTAS | MAXIMUM<br>ERRROP | STANDARD<br>ERRROP | PERCENT<br>STANDARD<br>ERROR | BEST FIT LINE |        |
|------------------|--------------------------------|-------------------|---------|-----------------|-------------------|--------------------|------------------------------|---------------|--------|
|                  |                                |                   |         |                 |                   |                    |                              | CORRFI.       | Coeff. |
| 0 -              | 1                              | 272               | .740    | .763            | .023              | 3.103              | 1.951                        | .300          | .446   |
| 1 -              | 3                              | 287               | 2.002   | 1.198           | -.904             | -40.170            | -2.366                       | .448          | .292   |
| 3 -              | 8                              | 238               | 5.432   | 5.613           | .181              | 3.329              | 11.050                       | 1.317         | .4504  |
| 8 -              | 22                             | 235               | 15.045  | 18.761          | 3.715             | 24.692             | 32.968                       | 2.931         | .305   |
| 22 -             | 50                             | 277               | 35.955  | 47.867          | 11.912            | 33.131             | 37.841                       | 4.934         | .468   |
| 50 -             | 99                             | 272               | 69.835  | 80.758          | 10.923            | 15.640             | 58.822                       | 10.878        | .469   |
| 99 -             | 179                            | 133               | 130.000 | 121.537         | -.8.463           | -6.510             | 102.832                      | 18.555        | .291   |
| 179 -            | 304                            | 60                | 225.123 | 188.286         | -76.837           | -16.363            | 233.231                      | 36.420        | .141   |
| 304 -            | 489                            | 33                | 371.122 | 306.406         | -64.715           | -17.438            | -211.356                     | 45.433        | .236   |
| ABOVE            | 489                            | 19                | 663.934 | 570.085         | -93.870           | -14.138            | -188.418                     | 67.861        | .876   |
| ABOVE            | 50                             | 517               | 146.400 | 136.114         | -.8.286           | -5.738             | 233.231                      | 45.273        | 1.113  |

AREAL WATER YEAR SUMMARY  
AREA NUMBER 1 YAMHILL NR MBP  
WATER YEAR 1963

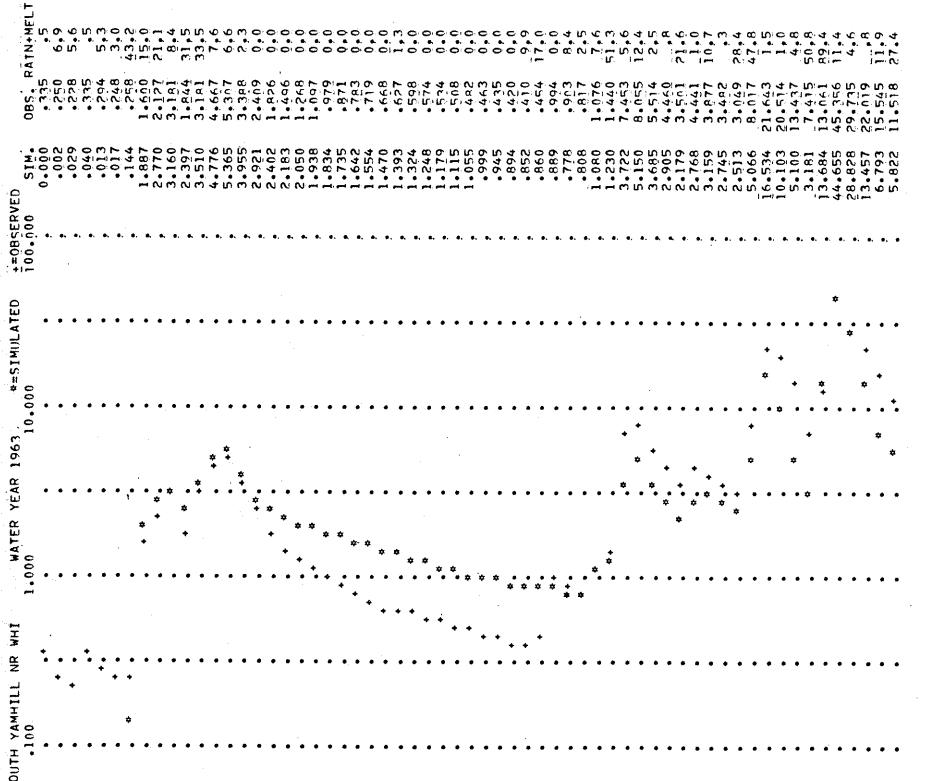
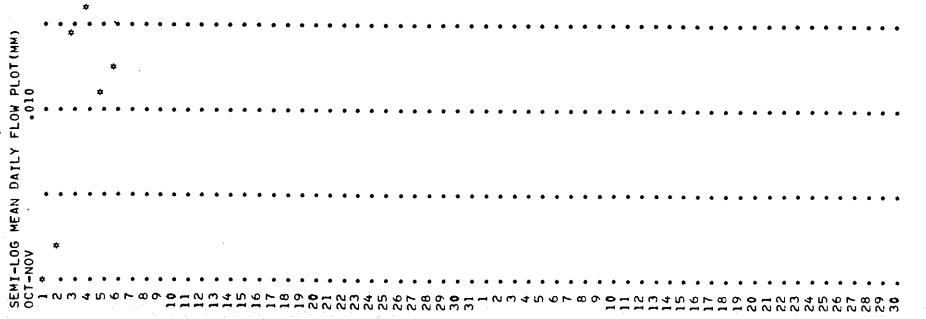
## SOIL MOISTURE ACCOUNTING VOLUMES

| MONTH        | TOTAL-PO      | IMPV-RO     | DIRECT-RO   | SURF-RO      | INTERFLW     | RASFFLOW     | CHANNEL    | NON-CHANFL    | RAIN+MFET    | POTENTIAL-ET | ACTUAL-ET |
|--------------|---------------|-------------|-------------|--------------|--------------|--------------|------------|---------------|--------------|--------------|-----------|
| OCT          | 56.5          | 1.9         | .4          | 0.0          | 16.1         | 38.8         | 0.0        | 192.3         | 72.0         | 63.0         |           |
| NOV          | 202.6         | 4.3         | 3.1         | 80.3         | 53.9         | 61.5         | 0.0        | 431.5         | 58.3         | 54.9         |           |
| DEC          | 138.1         | 1.2         | .9          | 0.0          | 27.9         | 108.3        | 0.0        | 123.4         | 33.2         | 32.8         |           |
| JAN          | 79.8          | 1.2         | 1.0         | 8.0          | 14.7         | 55.1         | 0.0        | 115.1         | 17.4         | 17.3         |           |
| FEB          | 168.8         | 1.7         | 1.6         | 23.5         | 42.1         | 100.0        | 0.0        | 174.0         | -15.4        | 15.3         |           |
| MAR          | 144.9         | 2.3         | 2.0         | 17.9         | 45.0         | 78.1         | 0.0        | 227.1         | 31.9         | 31.7         |           |
| APR          | 175.1         | 1.7         | 1.2         | 0.0          | 39.8         | 132.9        | 0.0        | 168.1         | 53.0         | 52.4         |           |
| MAY          | 104.7         | 1.0         | .8          | 0.0          | 20.8         | 82.9         | 0.0        | 104.9         | 76.7         | 70.2         |           |
| JUNE         | 19.1          | .4          | 0.0         | 0.0          | 0.0          | 19.5         | 0.0        | 37.8          | 83.6         | 60.4         |           |
| JULY         | 5.0           | .3          | 0.0         | 0.0          | 0.0          | 5.6          | 0.0        | 34.0          | 93.2         | 67.2         |           |
| AUG          | 2.1           | .4          | 0.0         | 0.0          | 0.0          | 2.7          | 0.0        | 35.1          | 93.3         | 47.3         |           |
| SEPT         | 3.2           | .6          | 0.0         | 0.0          | .2           | 3.2          | 0.0        | 56.6          | 82.9         | 57.6         |           |
| <b>TOTAL</b> | <b>1099.9</b> | <b>17.0</b> | <b>10.9</b> | <b>129.7</b> | <b>260.5</b> | <b>688.7</b> | <b>0.0</b> | <b>1700.0</b> | <b>710.9</b> | <b>570.2</b> |           |

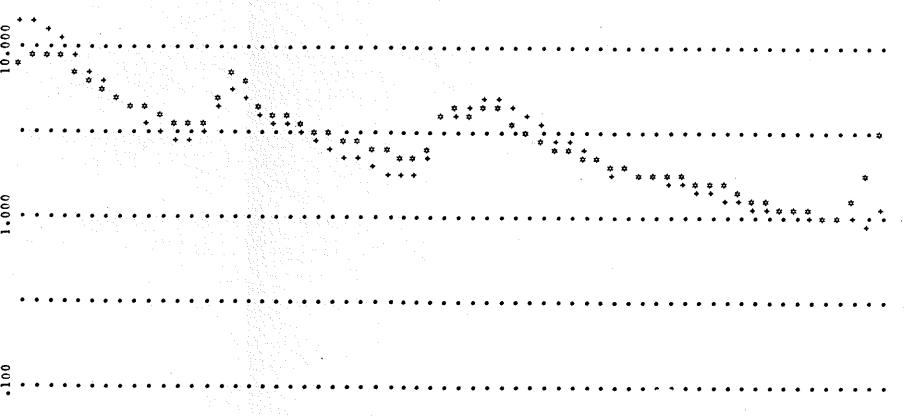
## SOIL MOISTURE VARIABLS AT END OF MONTH

| MONTH | UZTWC | LZFWC | LZTWC | LZFWC | L7FPC | L7DEFR | ADIMC | BALANC |
|-------|-------|-------|-------|-------|-------|--------|-------|--------|
| OCT   | 12.   | 0.    | 84.   | 17.   | 10.   | .69    | 115.  | -00    |
| NOV   | 35.   | 15.   | 140.  | 85.   | 26.   | .29    | 173.  | -00    |
| DEC   | 34.   | 4.    | 140.  | 46.   | 28.   | .39    | 173.  | -00    |
| JAN   | 35.   | 25.   | 140.  | 42.   | 28.   | .40    | 174.  | -00    |
| FEB   | 35.   | 3.    | 140.  | 52.   | 30.   | .37    | 175.  | -00    |
| MAR   | 35.   | 9.    | 140.  | 95.   | 32.   | .24    | 174.  | -00    |
| APR   | 32.   | 0.    | 138.  | 51.   | 31.   | .38    | 169.  | -00    |
| MAY   | 8.    | 0.    | 121.  | 21.   | 29.   | .51    | 128.  | -00    |
| JUN   | 20.   | 0.    | 87.   | 4.    | 27.   | .67    | 106.  | -00    |
| JULY  | 7.    | 0.    | 67.   | 1.    | 24.   | .74    | 73.   | -00    |
| AUG   | 17.   | 0.    | 45.   | 0.    | 22.   | .81    | 62.   | -00    |
| SEPT  | 19.   | 0.    | 40.   | 1.    | 21.   | .83    | 61.   | -00    |

L7DEFR IS THE LOWER ZONE SOIL MOISTURE DEFICIENCY RATIO.

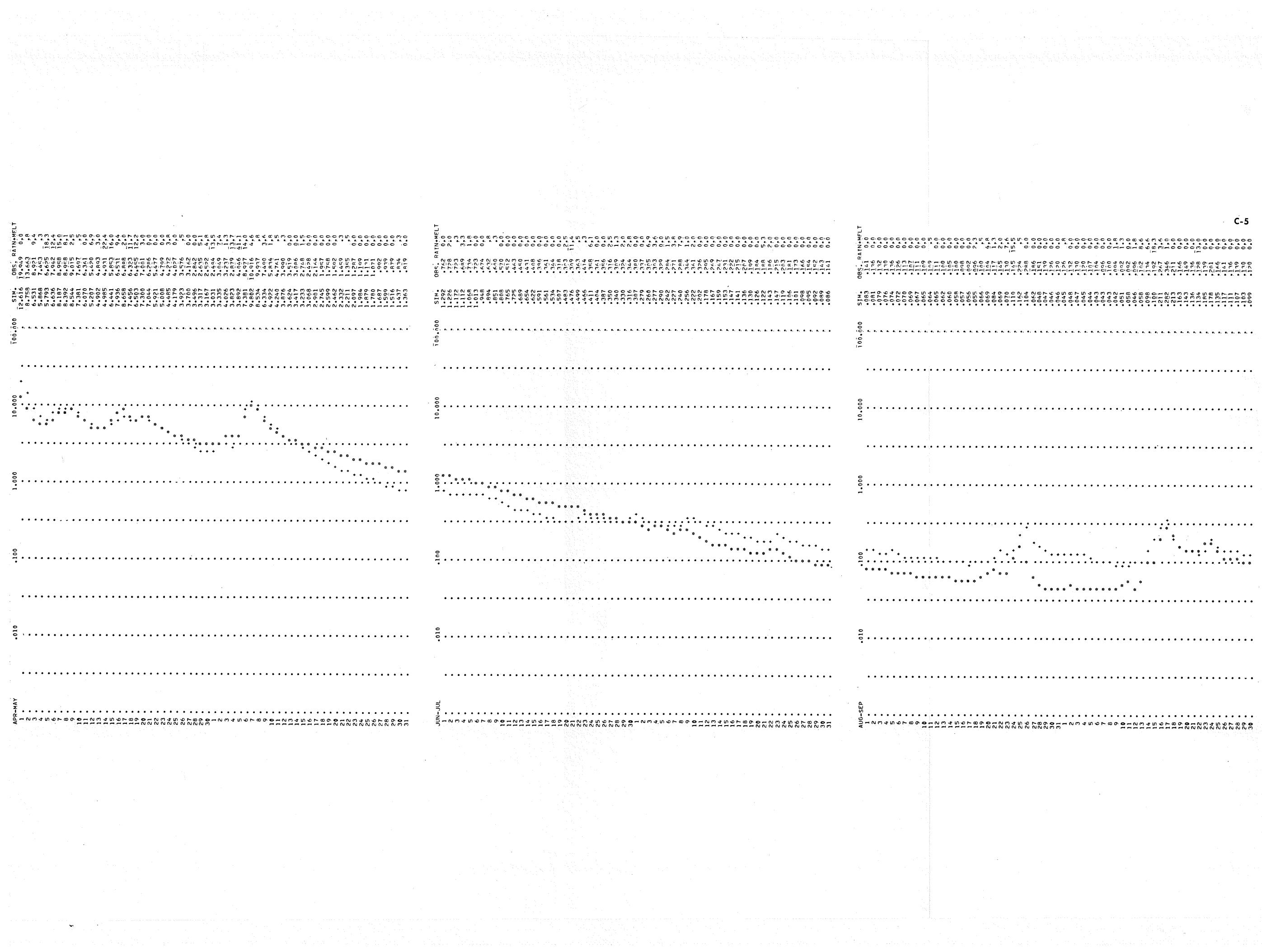


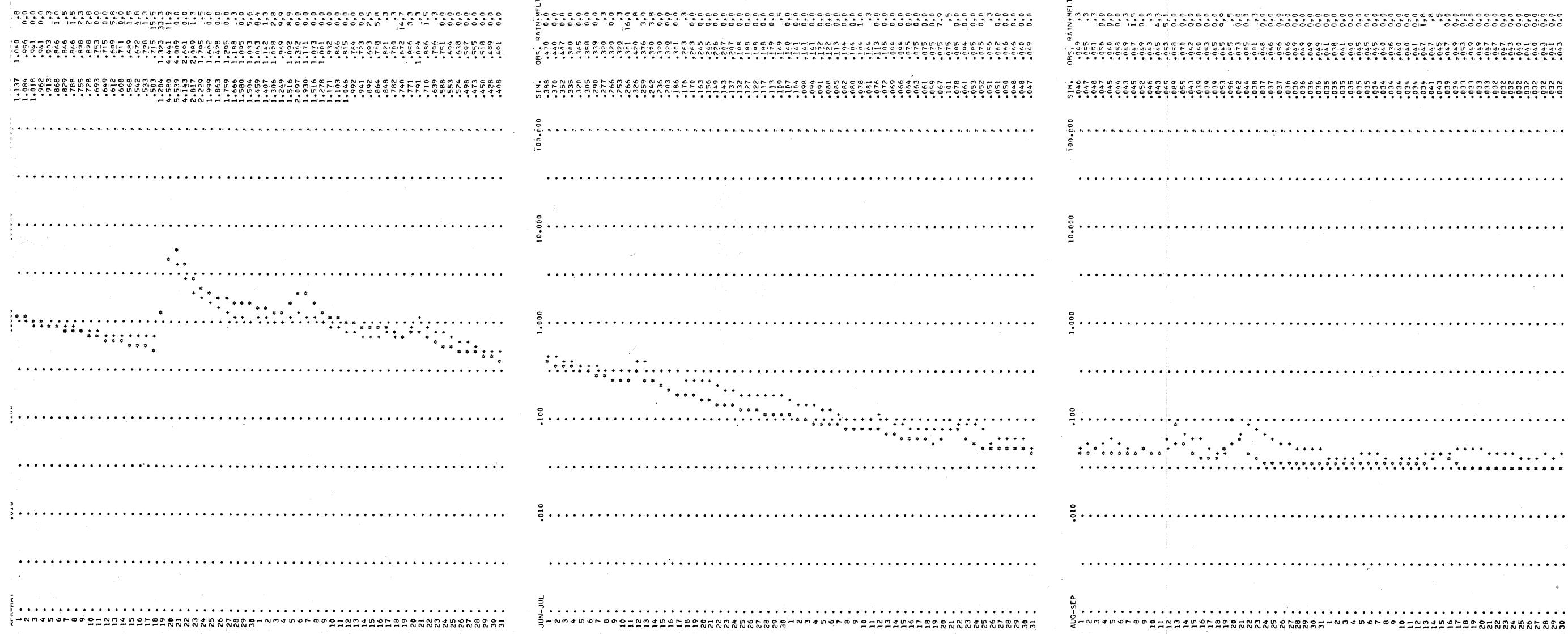
| OBS. RAIN+MELT |        | SIM. RAIN+MELT |      |
|----------------|--------|----------------|------|
| 8.256          | 8.062  | 7.9            | 7.9  |
| 8.783          | 13.743 | 5.6            | 5.6  |
| 8.746          | 12.779 | 6.1            | 6.1  |
| 8.679          | 11.465 | 2.0            | 2.0  |
| 7.745          | 9.169  | 5.6            | 5.6  |
| 6.048          | 7.453  | 0.0            | 0.0  |
| 1.680          | 1.076  | 2.5            | 2.5  |
| 1.230          | 1.440  | 6.1            | 6.1  |
| 3.722          | 7.453  | 5.6            | 5.6  |
| 5.150          | 8.055  | 12.4           | 12.4 |
| 3.685          | 5.514  | 2.5            | 2.5  |
| 2.905          | 4.450  | 1.0            | 1.0  |
| 2.179          | 3.551  | 21.6           | 21.6 |
| 2.168          | 4.441  | 1.0            | 1.0  |
| 3.159          | 3.877  | 10.7           | 10.7 |
| 2.745          | 3.492  | 3.3            | 3.3  |
| 2.113          | 3.049  | 28.4           | 28.4 |
| 5.066          | 8.017  | 47.8           | 47.8 |
| 16.334         | 21.643 | 1.5            | 1.5  |
| 10.033         | 20.514 | 1.0            | 1.0  |
| 5.000          | 13.437 | 4.6            | 4.6  |
| 3.381          | 7.915  | 50.8           | 50.8 |
| 13.684         | 13.061 | 89.4           | 89.4 |
| 14.655         | 45.756 | 11.4           | 11.4 |
| 28.928         | 22.019 | 4.6            | 4.6  |
| 13.457         | 15.545 | 11.8           | 11.8 |
| 6.793          | 11.518 | 27.4           | 27.4 |
| 5.822          | 11.518 | 27.4           | 27.4 |

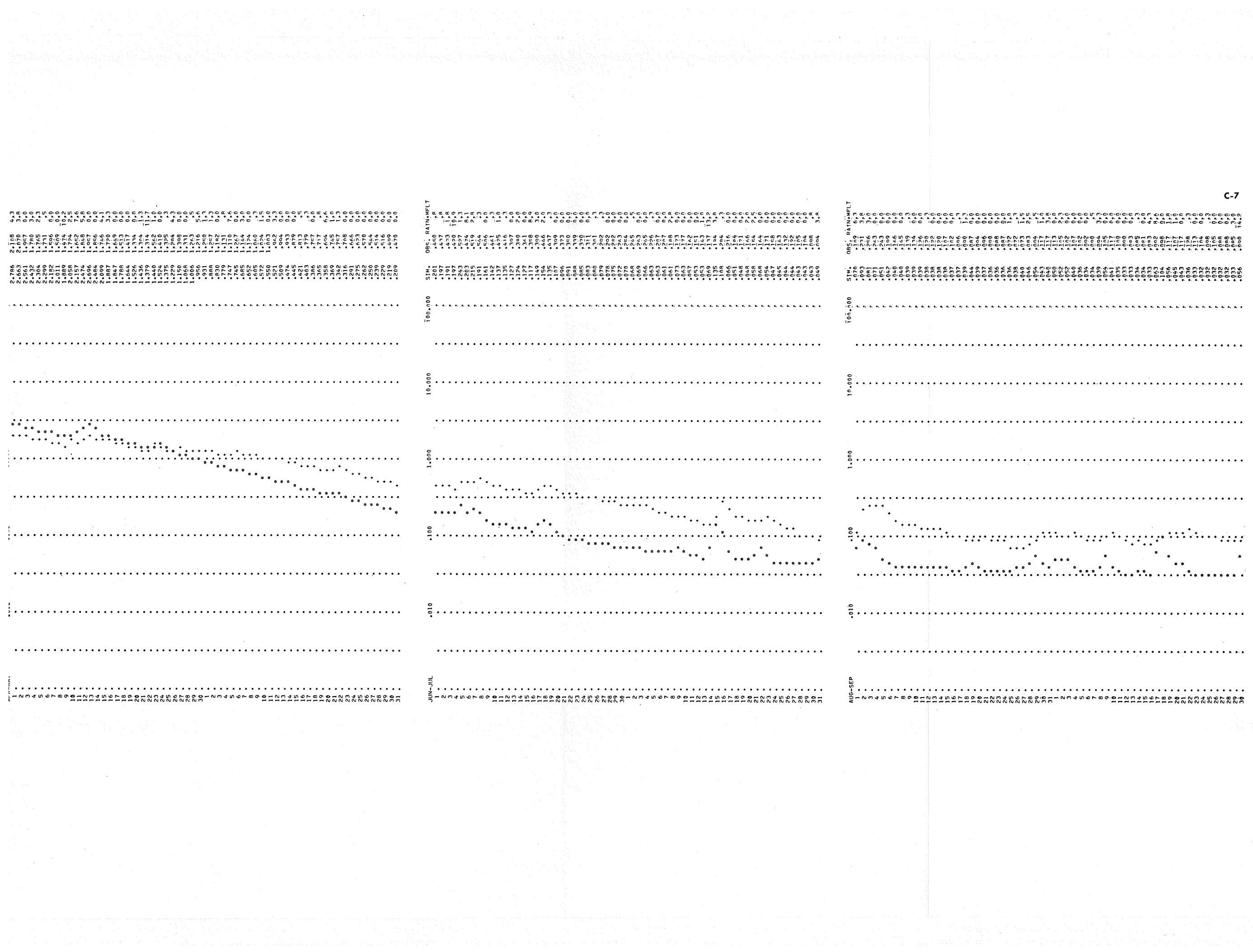


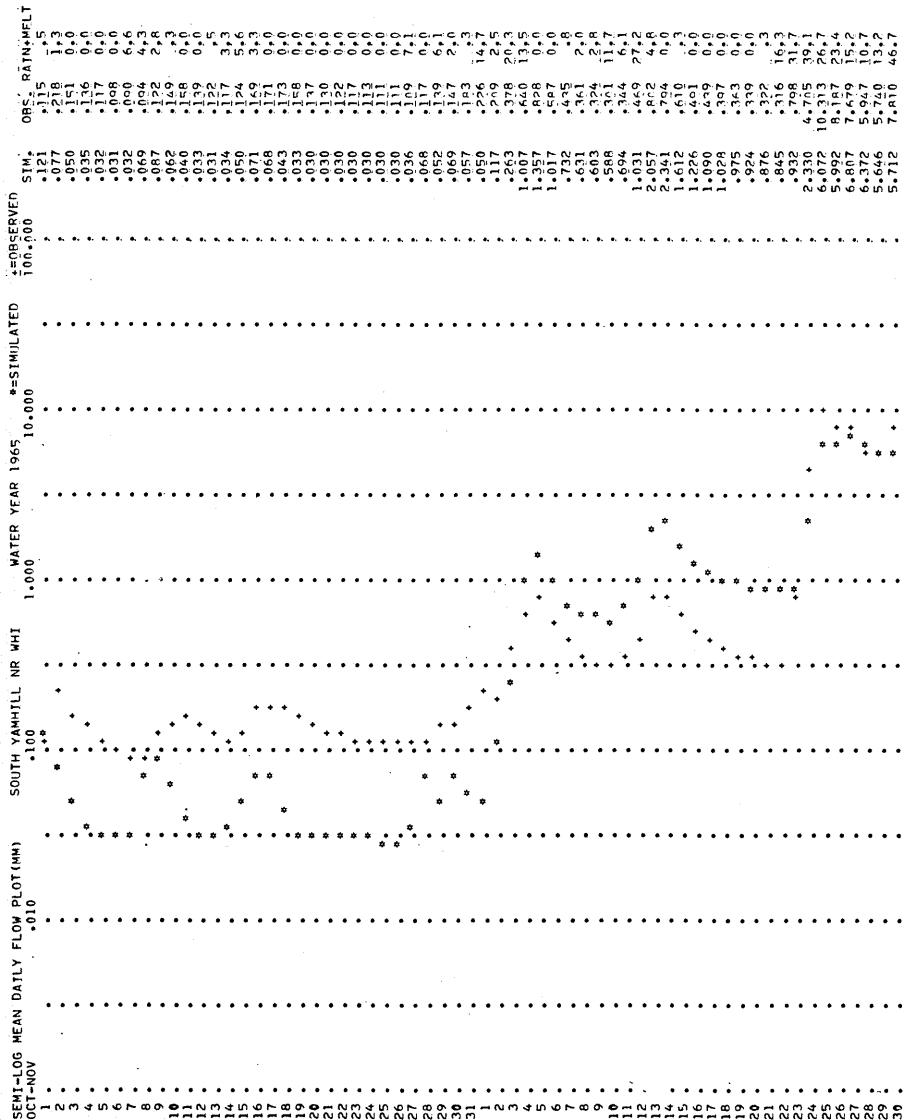
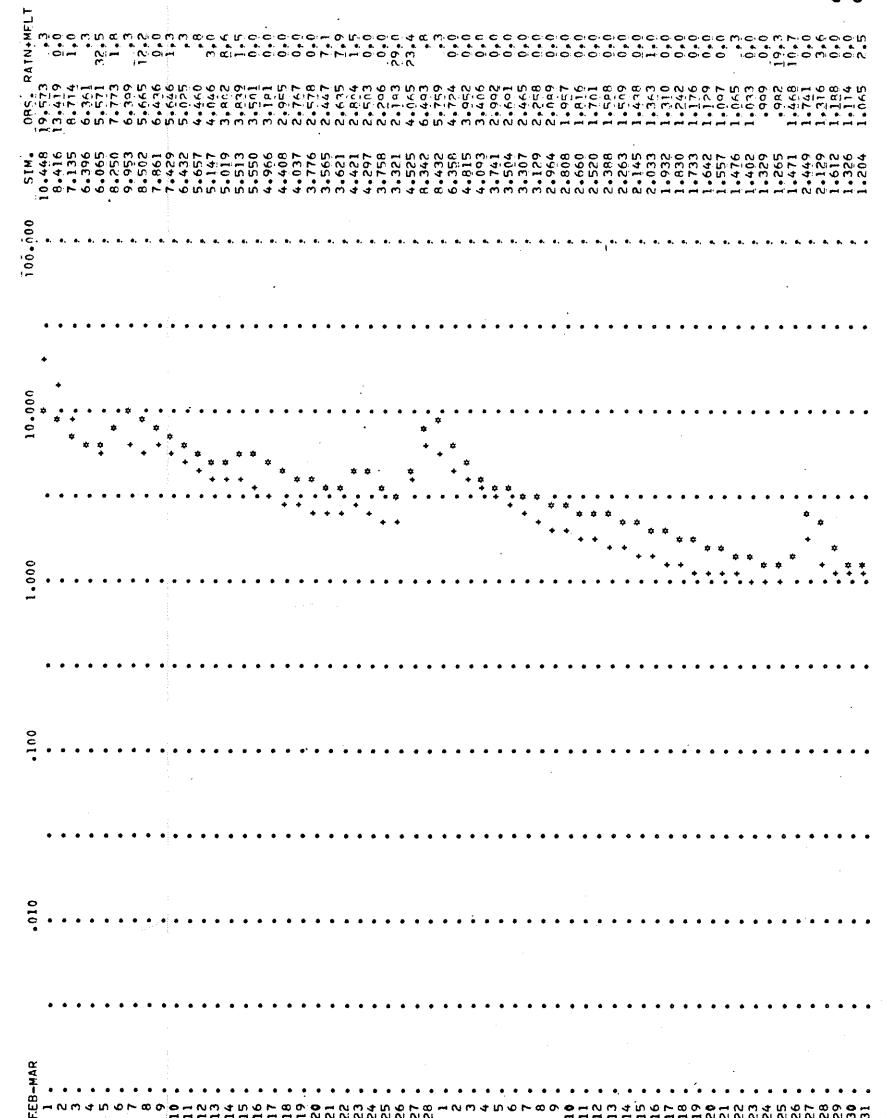
| OBS. RAIN+MELT |        | SIM. RAIN+MELT |      |
|----------------|--------|----------------|------|
| 8.256          | 8.062  | 7.9            | 7.9  |
| 8.783          | 13.743 | 5.6            | 5.6  |
| 8.746          | 12.779 | 6.1            | 6.1  |
| 8.679          | 11.465 | 2.0            | 2.0  |
| 7.745          | 9.169  | 5.6            | 5.6  |
| 6.048          | 7.453  | 0.0            | 0.0  |
| 1.680          | 1.076  | 2.5            | 2.5  |
| 1.230          | 1.440  | 6.1            | 6.1  |
| 3.722          | 7.453  | 5.6            | 5.6  |
| 5.150          | 8.055  | 12.4           | 12.4 |
| 3.685          | 5.514  | 2.5            | 2.5  |
| 2.905          | 4.450  | 1.0            | 1.0  |
| 2.179          | 3.551  | 21.6           | 21.6 |
| 2.168          | 4.441  | 1.0            | 1.0  |
| 3.159          | 3.877  | 10.7           | 10.7 |
| 2.745          | 3.492  | 3.3            | 3.3  |
| 2.113          | 3.049  | 28.4           | 28.4 |
| 5.066          | 8.017  | 47.8           | 47.8 |
| 16.334         | 21.643 | 1.5            | 1.5  |
| 10.033         | 20.514 | 1.0            | 1.0  |
| 5.000          | 13.437 | 4.6            | 4.6  |
| 3.381          | 7.915  | 50.8           | 50.8 |
| 13.684         | 13.061 | 89.4           | 89.4 |
| 14.655         | 45.756 | 11.4           | 11.4 |
| 28.928         | 22.019 | 4.6            | 4.6  |
| 13.457         | 15.545 | 11.8           | 11.8 |
| 6.793          | 11.518 | 27.4           | 27.4 |
| 5.822          | 11.518 | 27.4           | 27.4 |

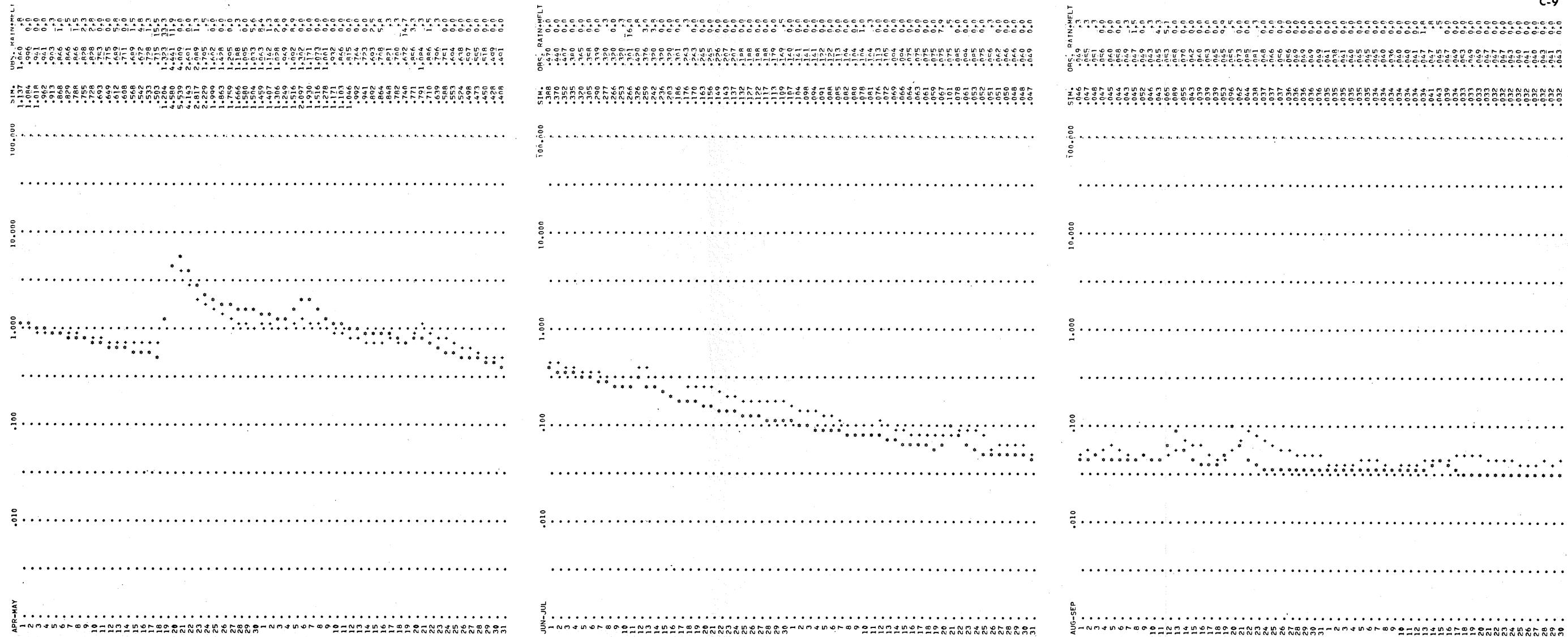
| OBS. RAIN+MELT |        | SIM. RAIN+MELT |      |
|----------------|--------|----------------|------|
| 8.460          | 8.361  | 13.2           | 13.2 |
| 12.206         | 11.161 | 5.3            | 5.3  |
| 18.507         | 22.267 | 16.0           | 16.0 |
| 23.159         | 33.459 | 15.0           | 15.0 |
| 13.021         | 22.554 | 0.0            | 0.0  |
| 4.819          | 5.522  | 0.0            | 0.0  |
| 4.359          | 4.611  | 0.0            | 0.0  |
| 4.055          | 3.877  | 0.0            | 0.0  |
| 3.818          | 3.764  | 8.4            | 8.4  |
| 6.109          | 4.667  | 6.3            | 6.3  |
| 6.241          | 4.912  | 0.0            | 0.0  |
| 5.342          | 4.366  | 0.0            | 0.0  |
| 4.226          | 3.362  | 0.0            | 0.0  |
| 3.662          | 3.331  | 0.0            | 0.0  |
| 3.342          | 2.974  | 0.0            | 0.0  |
| 3.143          | 2.768  | 15.0           | 15.0 |
| 3.579          | 3.915  | 3.6            | 3.6  |
| 4.559          | 3.773  | 0.0            | 0.0  |
| 4.052          | 3.463  | 4.6            | 4.6  |
| 3.556          | 3.775  | 7.4            | 7.4  |
| 4.78           | 3.990  | 9.7            | 9.7  |
| 3.342          | 2.974  | 0.0            | 0.0  |
| 3.143          | 2.768  | 15.0           | 15.0 |
| 3.639          | 3.331  | 0.0            | 0.0  |
| 3.175          | 1.920  | 11.2           | 11.2 |
| 2.311          | 2.051  | 3.0            | 3.0  |
| 2.948          | 2.049  | 0.0            | 0.0  |
| 2.633          | 1.957  | 1.0            | 1.0  |
| 3.328          | 2.296  | 11.7           | 11.7 |
| 6.062          | 6.163  | 3.0            | 3.0  |
| 1.941          | 1.977  | 3.0            | 3.0  |
| 1.855          | 1.724  | 12.4           | 12.4 |
| 1.950          | 2.070  | 0.0            | 0.0  |
| 2.661          | 2.070  | 5.5            | 5.5  |
| 2.888          | 1.995  | 6.1            | 6.1  |
| 2.776          | 1.957  | 15.5           | 15.5 |
| 3.328          | 2.296  | 11.7           | 11.7 |
| 6.062          | 6.163  | 3.0            | 3.0  |
| 1.941          | 1.977  | 3.0            | 3.0  |
| 1.             |        |                |      |

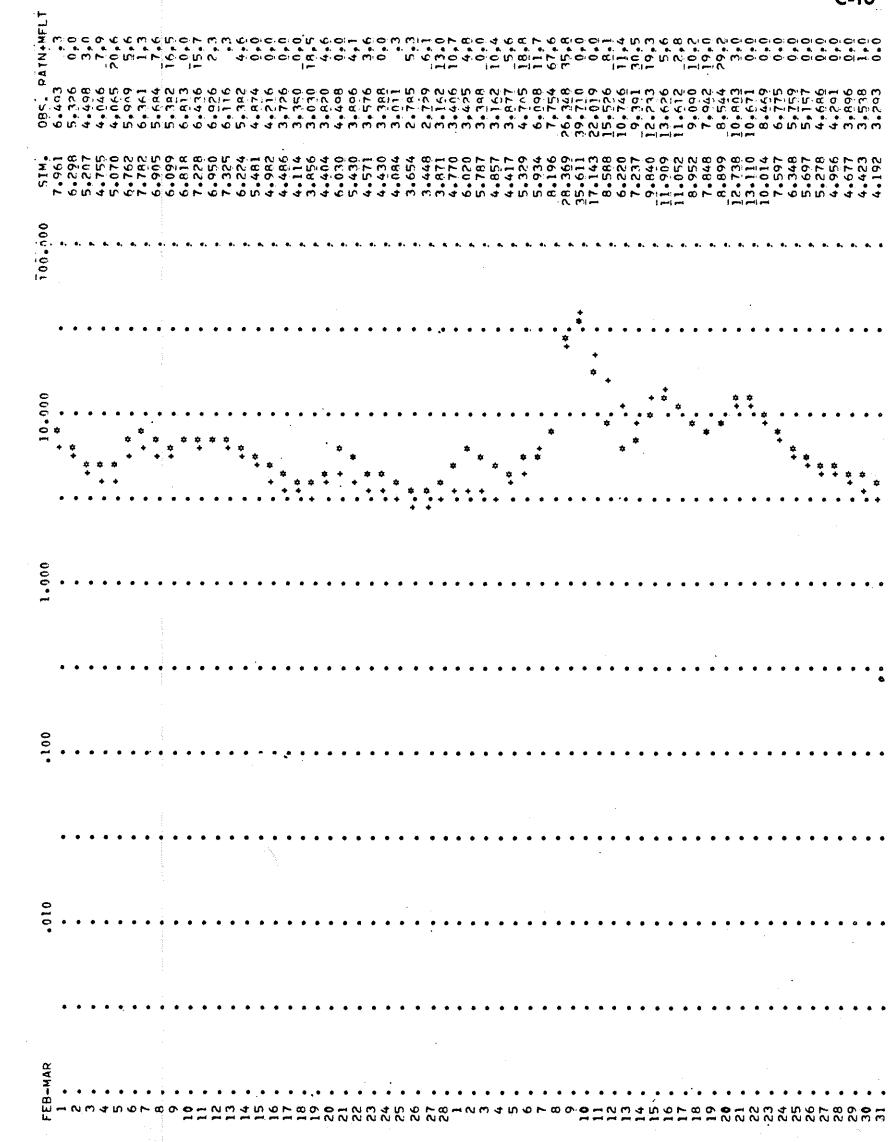
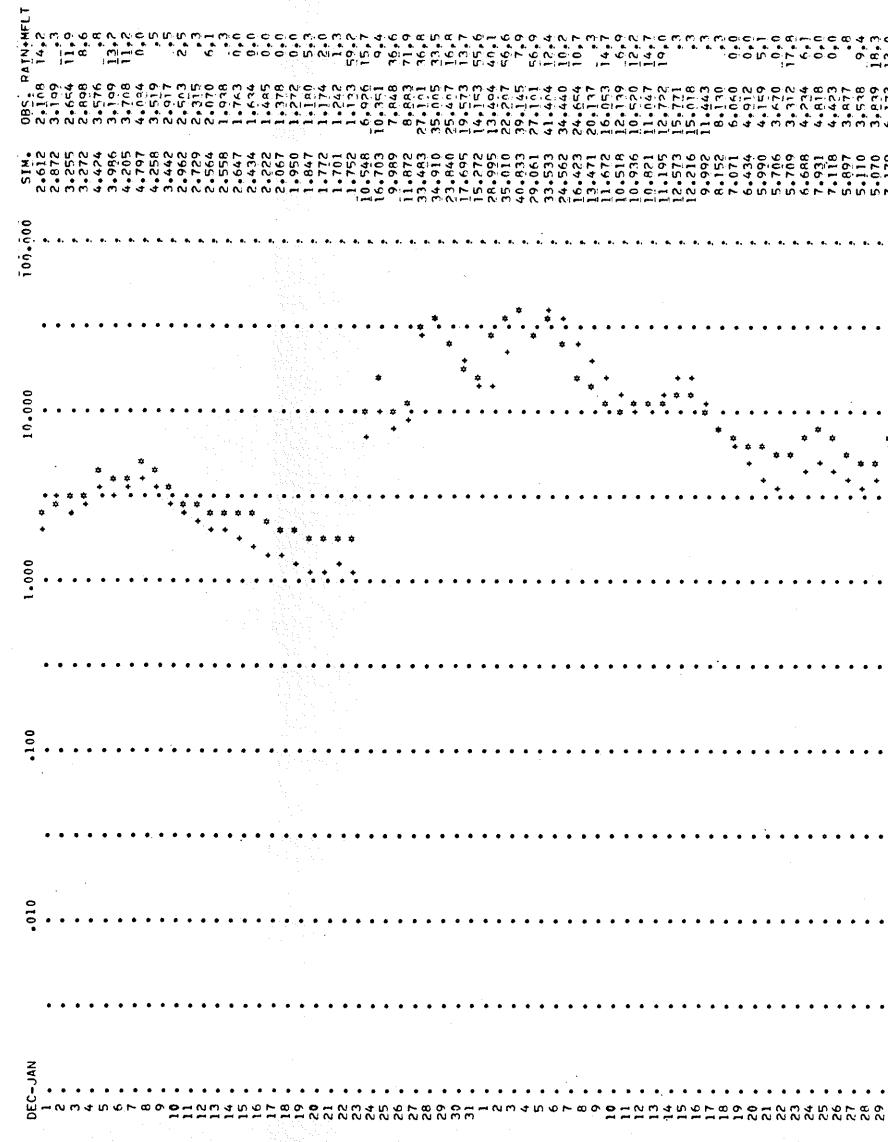
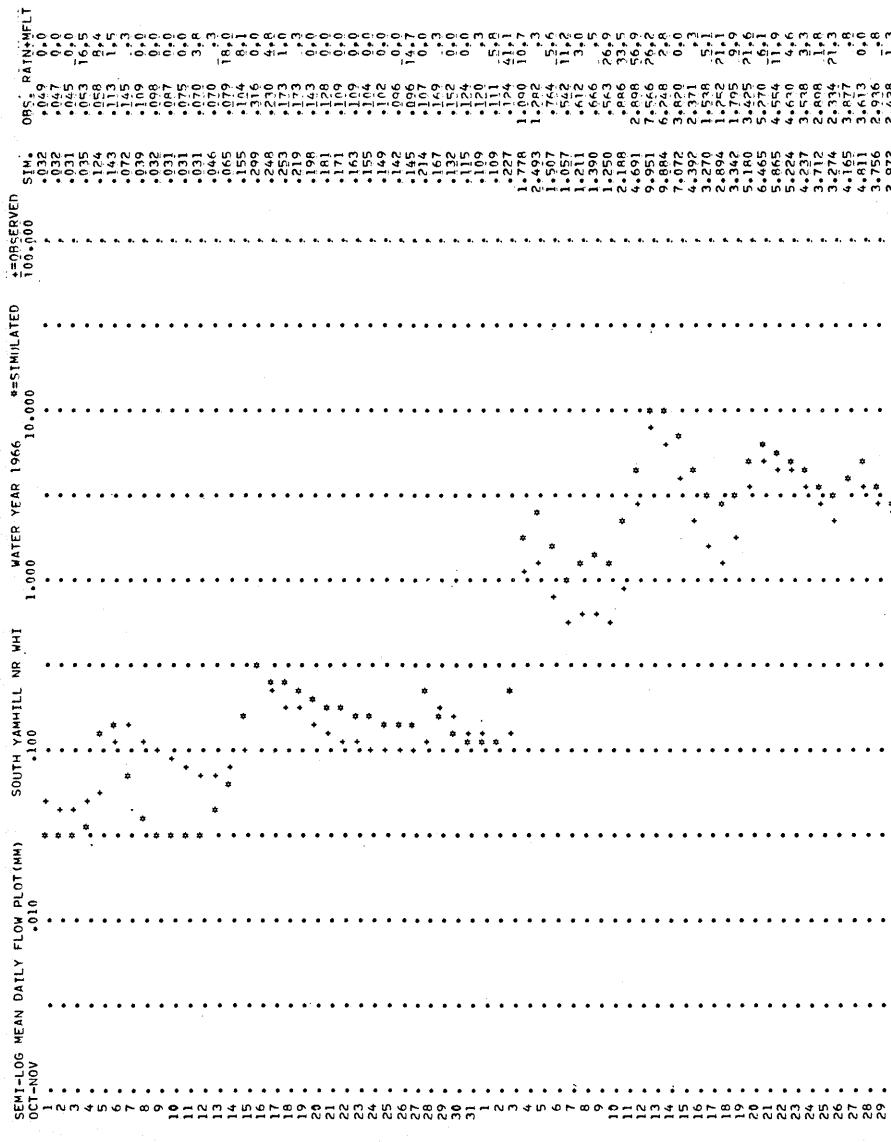




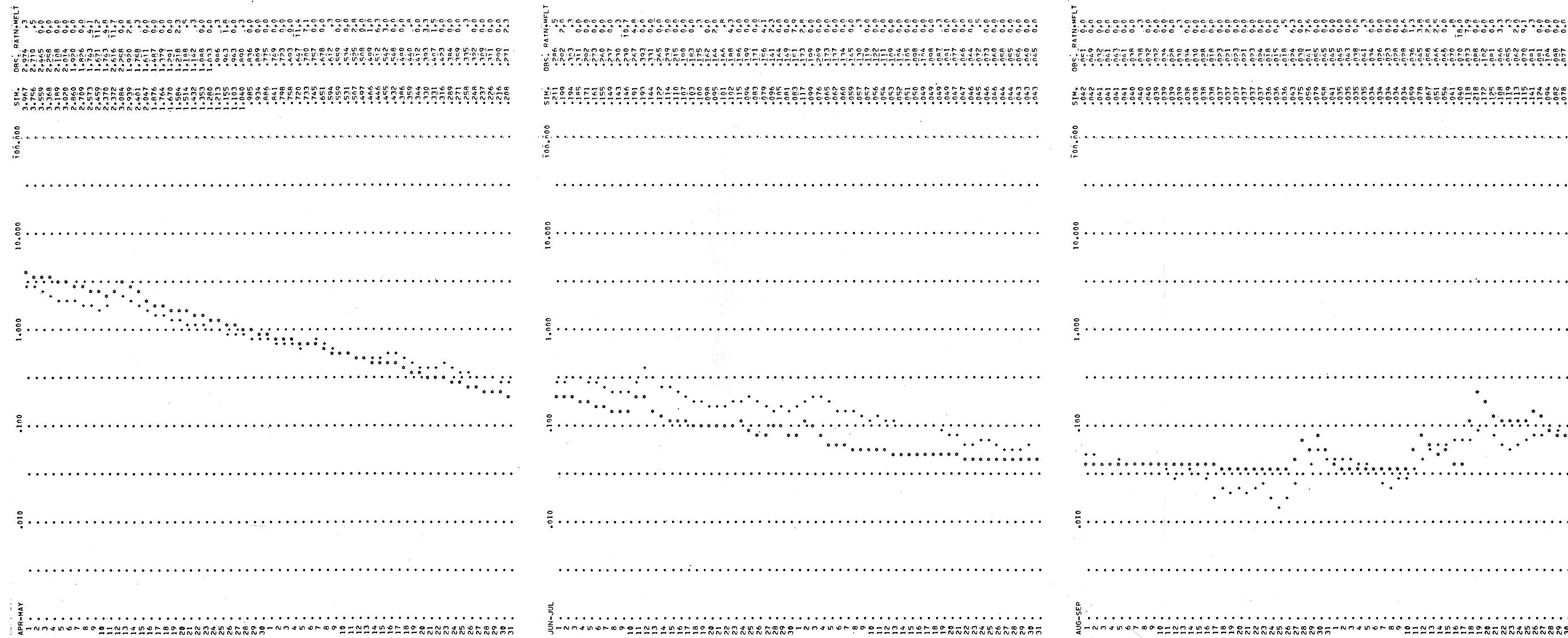


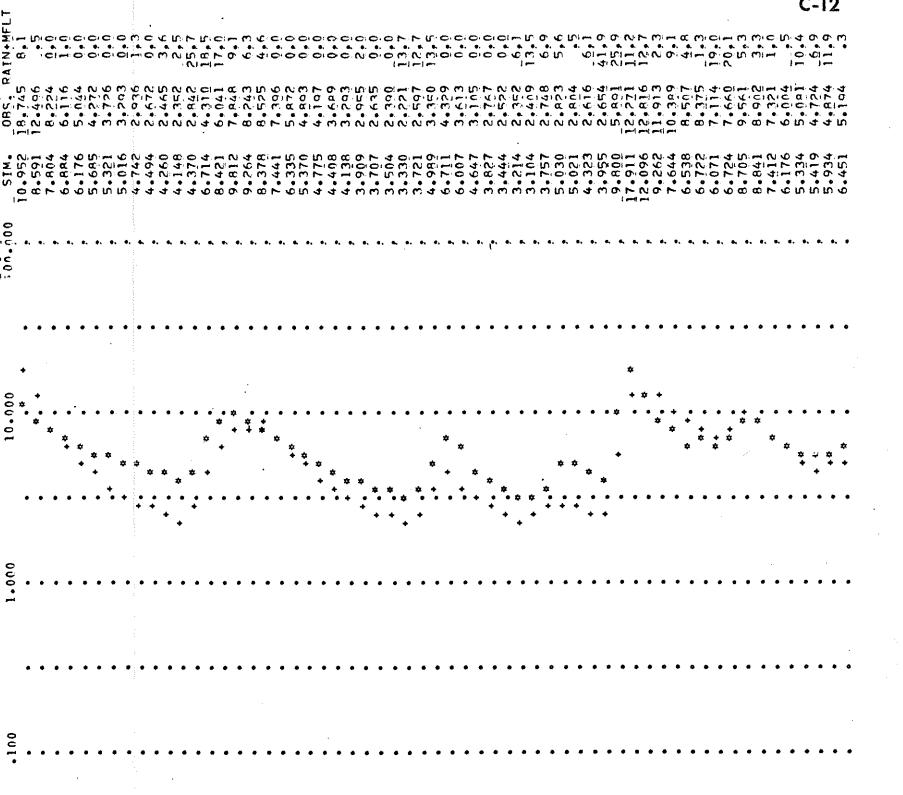
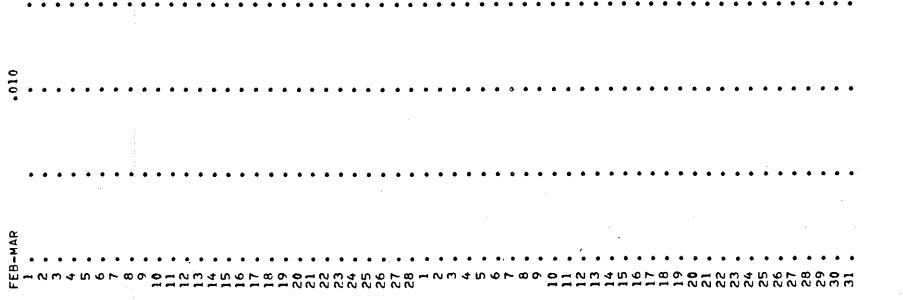
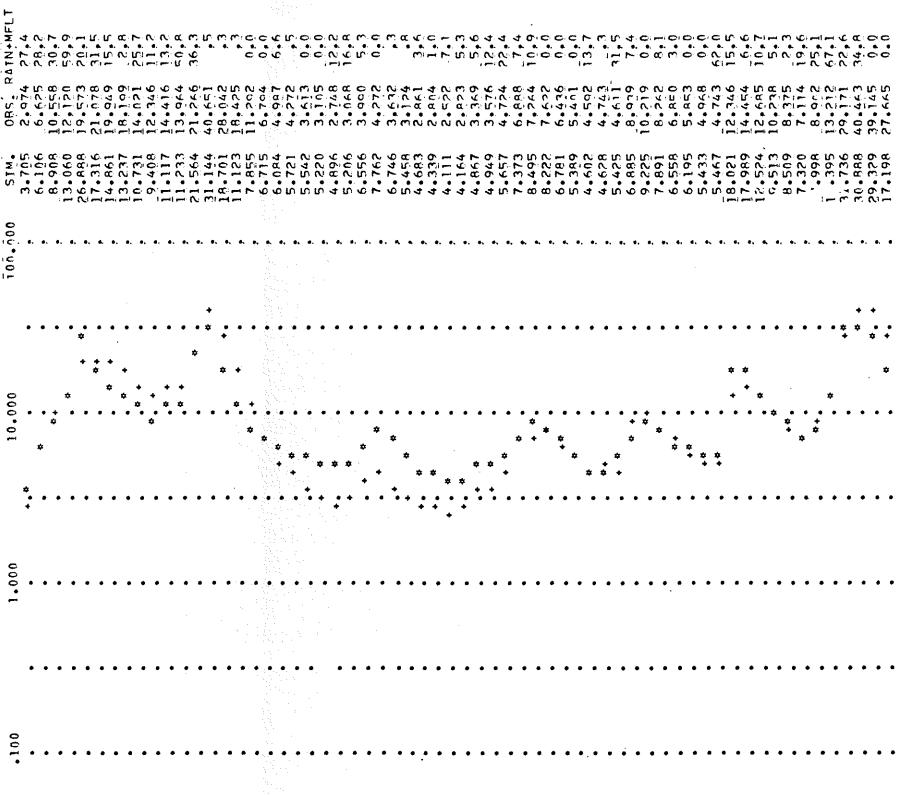
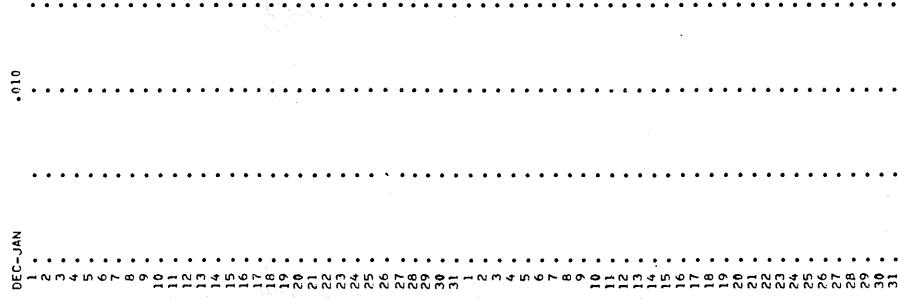
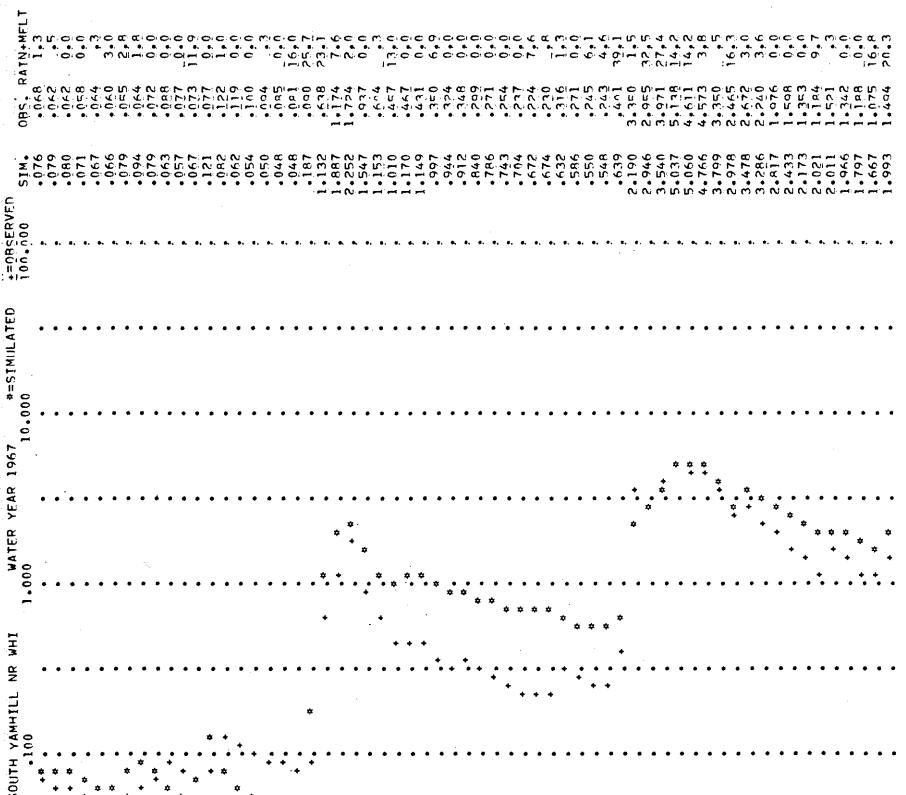
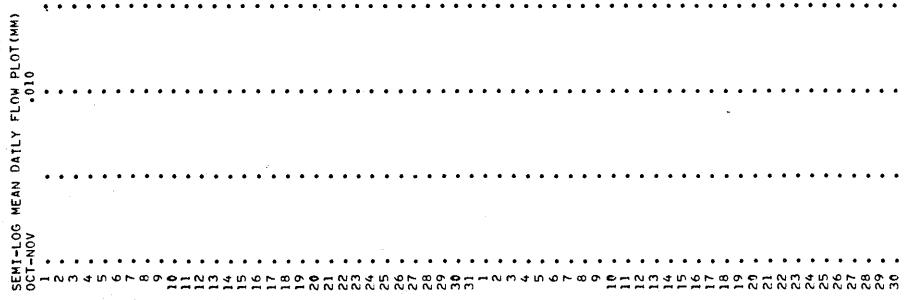


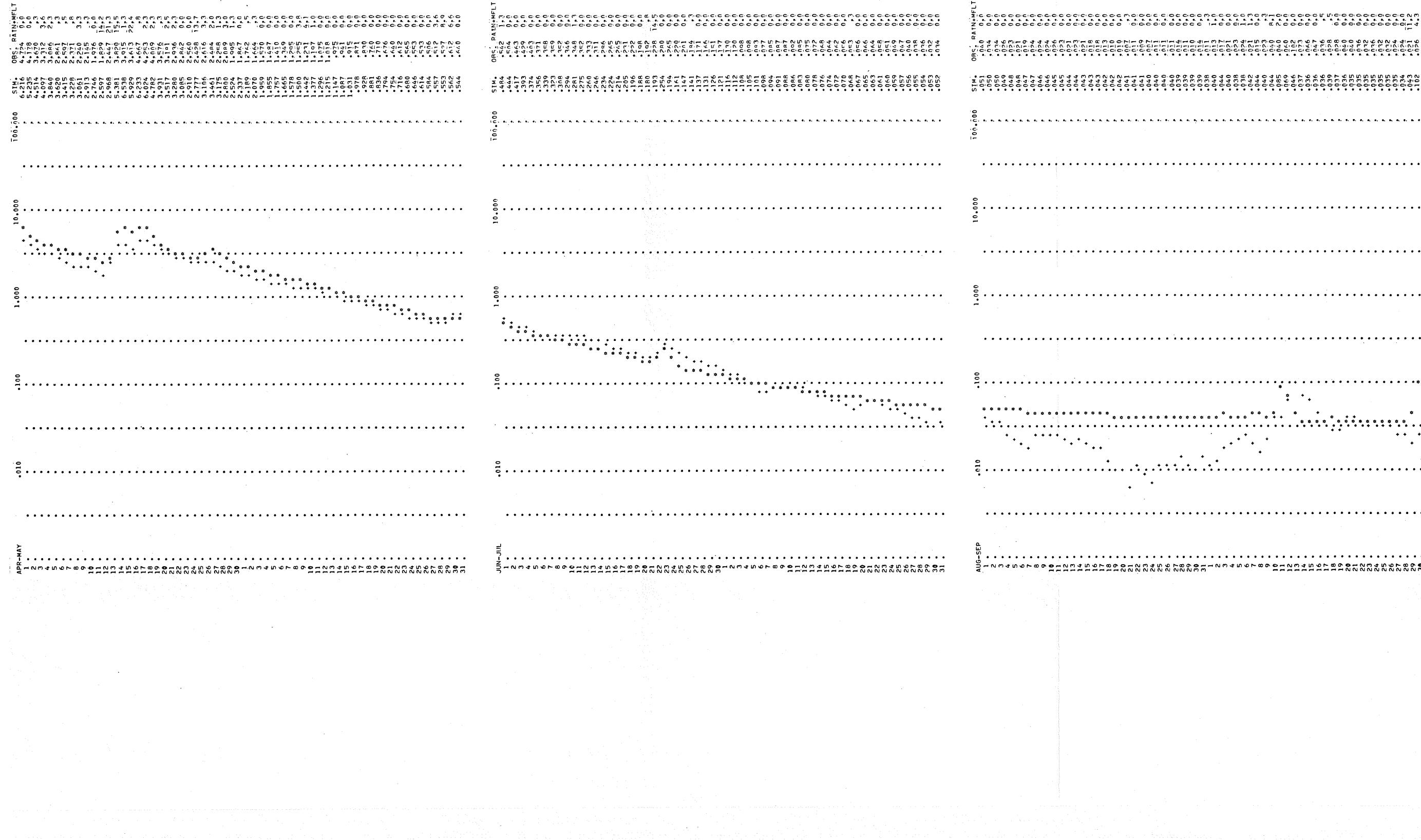




C-10







## APPENDIX D

## LISTING OF THE NWSRFS SOIL MOISTURE ACCOUNTING SUBROUTINE

```
SUBROUTINE LAND(ID1,IP1, ID2, IP2,MOSM,ICOUNT,IRG)
```

```
C*****  
C  
C NWSRFS SOIL MOISTURE ACCOUNTING PROCEDURE  
C BASED ON SOIL MOISTURE ACCOUNTING IN THE SACRAMENTO MODEL  
C  
C*****
```

```
C LAND VARIABLES
```

```
C REAL LZTWC,LZFPC,LZFSC,LZTWC1,LZFPC1,LZFSC1,LZTWM,LZFPM,LZFSM,LZPK  
1,LZSK
```

```
C DIMENSION MOSM(8,2),EPDIST(4)
```

```
C GENERAL PROGRAM VARIABLES
```

```
C INTEGER ROUTE,SNOW,SNOWA,YRIN,YR1,STORE,YEAR,PLT6HR,SAVEFW,COMPAR,  
1PTEST,PLOT,CTEST,SIXIN,DRSER,STDA,STP6,YR2,STAT,PEG  
REAL INFRO
```

```
C COMMON /G/ MONTH,MOIN,LAST,ROUTE,NGAGES,SNOW,SNOWA(12),YRIN,NPEGS,  
1YR1,NPTS,STORE,BASIN(20),YEAR,SSF(3,12),SUF(3,12),PLT6HR,SAVEFW,  
2COMPAR(3),PTEST,PLUT(3),LINEP,INFRO(20),PLUTMX(3),CTEST,FSFLOW(3),  
3PEG(5),STAT,YR2,AREA(6),SIXIN(3),DRSER(3),STDA(2,10),STP6(2,10),  
4IYFAR1(3),IPT,METRIC(3),NQ24,NQ6,NPTSUP,IQ24IN(3),IN6IN(3)
```

```
C SOIL MOISTURE ACCOUNTING VARIABLES.
```

```
C COMMON/SOIL/BAL(5),PL(5,18),VL(5,6),SL(5,10),E(5,12,31)
```

```
C TIME SERIES IDENTIFICATIONS AND DESCRIPTIONS.
```

```
C COMMON /TSID/ AID(5,3),ANAME(5,5),PEID(3,3),FPNAME(3,5),FPID(3,3),  
1Q24ID(3,3),Q6ID(3,3),UPFWID(3,3),PXID(5,3)
```

```
C BASIC DATA ARRAYS
```

```
C COMMON /BD/ PX(5,4,31),TA(5,4,31),PE(3,31),RD(5,4,31),OFW6(3,4,31)  
1,SEFW6(3,4,31),UFW6(3,4,31),OFW24(3,31)
```

```
C SNOW AND LAND COMMON BLOCK
```

```
C COMMON/SL/COVER(5,31),EFC(5),PXADJ(5),NTAG,NWEG  
DATA EPDIST/0.0,0.33,0.67,0.0/
```

```
C*****  
C  
C IPRINT=0  
C IF((MONTH.EQ.0,MOSM(ICOUNT,1)).AND.(YEAR.EQ.MOSM(ICOUNT,2)))IPRINT=1  
C IF(IPRINT.EQ.0) GO TO 200
```

```
C PRINT 900,MONTH,YEAR,(ANAME(IRG,I),I=1,5)
```

```
900 FORMAT(1H,33HSIX-HOUR SOIL MOISTURE OUTPUT FOR,1X,I2,1H/,14,2X,5A  
14,20X,39HUNITS OF ALL QUANTITIES ARE MILLIMETERS)
```

```
C PRINT 902
```

```
902 FORMAT(1H ,5X,19HPERC IS PERCOLATION,5X,31HBASEFW IS THE CHANNEL C  
1OMPONENT,5X,67HTOTAL-RD IS CHANNEL INFLOW MINUS ET FROM THE AREA D)  
2EFINED BY SARVA.)
```

```
C PRINT 901
```

```
901 FORMAT(1H ,3HDAY,1X,2HPD,2X,5HUZTWC,2X,5HUZFPC,2X,5HLZTWC,2X,5HLZF  
1SC,2X,5HLZFPC,2X,5HADIMC,4X,4HPERC,1X,7HIMPV-RD,2X,6HDIRECT,2X,6HS  
2UR-RD,1X,7HINTERFW,2X,6HBASEFW,1X,8HTOTAL-RD,1X,7HET-DEMD,1X,6HACT  
3-ET,2X,9HRAIN+MFILT)
```

```

C..... .
C
200 SRDT=0.0
    SIMPVT=0.0
    SRODT=0.0
    SROST=0.0
    SINTFT=0.0
    SGWFT=0.0
    SRECHT=0.0
    SETT=0.0
    SPRT=0.0
    SPET=0.0
C
C      INITIAL VALUES OF VARIABLES
C
UZTWC=VL(IRG,1)
UZFWC=VL(IRG,2)
LZTWC=VL(IRG,3)
LZFPC=VL(IRG,5)
LZFSC=VL(IRG,4)
ADIMC=VL(IRG,6)
UZTWC1=UZTWC
UZFWC1=UZFWC
C
LZTWC1=LZTWC
LZFPC1=LZFPC
LZFSC1=LZFSC
C
ADIMC1=ADIMC
C
C      INITIAL VALUES OF PARAMETERS
C
PPADJ=PL(IRG,1)
PFADJ=PL(IRG,2)
UZTWM=PL(IRG,3)
UZFWM=PL(IRG,4)
UZK=PL(IRG,5)
ZPERC=PL(IRG,9)
REXP=PL(IRG,10)
PCTIM=PL(IRG,6)
ADIMP=PL(IRG,7)
SARVA=PL(IRG,8)
LZTWM=PL(IRG,11)
LZFPM=PL(IRG,13)
LZFSM=PL(IRG,12)
LZPK=PL(IRG,15)
LZSK=PL(IRG,14)
PFREE=PL(IRG,16)
RSERV=PL(IRG,17)
SIDF=PL(IRG,18)
C
WATSF=SARVA
SARRA=0.0
C
IF(SARVA.LE.PCTIM) GO TO 201
WATSF=PCTIM
SARRA=SARVA-PCTIM
C
201 IGPE=PEG(IRG)
EFCT=EFC(IRG)
SAVED=RSERV*(LZFPM+LZFSM)
PAREA=1.0-PCTIM-ADIMP
IP6=IPI
IDA=IDI
GO TO 204
C*****

```

```

C      BEGINNING OF 6 HOUR AND DAY LOOP
C*****
C
C      205 IF(IP6.NE.1) GO TO 210
C      204 IF(IGPE.GT.0) GO TO 206
C          NO PE INPUT, THUS PE IS OBTAIN FROM MEAN SEASONAL CURVE.
C          EP=E(IRG,MONTH,IDA)
C          GO TO 207
C
C      DAILY PE TIME SERIES IS AVAILABLE
C
C      206 EP=PE(IGPE,IDA)
C          EP=EP*E(IRG,MONTH,IDA)
C      207 EP=EP*PEADJ
C          SPET=SPFT+EP
C
C          IF(SNOW.EQ.1) EP=EFCT*EP+(1.0-EFCT)*(1.0-COVER(IRG,IDA))*EP
C      210 IF((SNOW.F0.1).AND.(SNOWA(MONTH).EQ.1)) GO TO 219
C          PX6 = PX(IRG,IP6,IDA)*PPADJ
C          GO TO 215
C
C          IF SNOW IS BEING CONSIDERED, PXADJ HAS ALREADY BEEN APPLIED
C
C      219 PX6 = PX(IRG,IP6,IDA)
C      215 SPRT=SPRT+PX6
C
C          PX6 IS THE SIX HOUR RAINFALL OR SNOW COVER OUTFLOW
C*****
C
C      EDMND IS SIX-HOUR EVAPORATION DEMAND
C      EDMND=EP*EPDIST(IP6)
C
C.....*
C      E1=EDMND*(UZTWC/UZTWM)
C      RED=EDMND-E1
C
C      RED IS RESIDUAL EVAP DEMAND
C
C      UZTWC=UZTWC-E1
C      E2=0.0
C      IF(UZTWC.GE.0.) GO TO 220
C
C      E1 CAN NOT EXCEED UZTWC
C
C      E1=E1+UZTWC
C      UZTWC=0.0
C      RED=EDMND-E1
C      IF(UZFWC.GE.RED) GO TO 221
C
C.....*
C
C      E2 IS EVAP FROM UZFWC.
C
C      E2=UZFWC
C      UZFWC=0.0
C      RED=RED-E2
C      GO TO 225
C
C      221 E2=RED
C      UZFWC=UZFWC-E2
C      RED=0.0
C
C      220 IF((UZTWC/UZTWM).GE.(UZFWC/UZFWM)) GO TO 225
C

```

```

C.....  

C      UPPER ZONE FREE WATER RATIO EXCEEDS UPPER ZONE  

C      TENSION WATER RATIO, THUS TRANSFER FREE WATER TO TENSION  

C      UZRAT=(UZTWC+UZFWM)/(UZTWM+UZFWM)  

C      UZTWC=UZTWM*UZRAT  

C      UZFWM=UZFWM*UZRAT  

C.....  

C      COMPUTE ET FROM ADIMP AREA--E5  

C      225 E5=E1+(RED+E2)*((ADIMC-E1-UZTWC)/(UZTWM+LZTWM))  

C.....  

C      COMPUTE ET FROM LZTWC (E3)  

C      F3=RED*(LZTWC/(UZTWM+LZTWM))  

C      LZTWC=LZTWC-E3  

C      IF(LZTWC.GE.0.0) GO TO 226  

C      E3 CAN NOT EXCEED LZTWC  

C      E3=E3+LZTWC  

C      LZTWC=0.0  

C.....  

C      226 RATLZT=LZTWC/LZTWM  

C      RATLZ=(LZTWC+LZFPC+LZFSC-SAVED)/(LZTWM+LZFPM+LZFSM-SAVED)  

C      IF(RATLZT.GE.RATLZ) GO TO 230  

C      RESUPPLY LOWER ZONE TENSION WATER FROM LOWER  

C      ZONE FREE WATER IF MORE WATER AVAILABLE THERE.  

C      DEL=(RATLZ-RATLZT)*LZTWM  

C      TRANSFER FROM LZFSC TO LZTWC.  

C      LZTWC=LZTWC+DEL  

C      LZFSC=LZFSC-DEL  

C      IF(LZFSC.GE.0.0) GO TO 230  

C      IF TRANSFER EXCEEDS LZFSC THEN REMAINDER COMES FROM LZFPC  

C      LZFPC=LZFPC+LZFSC  

C      LZFSC=0.0  

C.....  

C      230 ROIMP=PX6*PCTIM  

C      ROIMP IS RUNOFF FROM THE MINIMUM IMPERVIOUS AREA.  

C      SIMPVT=SIMPVT+ROIMP  

C      ADJUST ADIMC, ADDITIONAL IMPERVIOUS AREA STORAGE, FOR EVAPORATION.  

C      ADIMC=ADIMC-E5  

C      IF(ADIMC.GE.0.0) GO TO 231  

C.....  

C      E5 CAN NOT EXCEED ADIMC.  

C      E5=E5+ADIMC  

C      ADIMC=0.0  

C      231 E5=E5*ADIMP  

C      E5 IS ET FROM THE AREA ADIMP.  

C      PAV=PX6+UZTWC-UZTWM  

C      PAV IS THE PERIOD AVAILABLE MOISTURE IN EXCESS  

C      OF UZTW REQUIREMENTS.

```

```

C      IF(PAV.GE.0.0) GO TO 232
C      ALL MOISTURE HELD IN UZTW--NO EXCESS.
C
C      UZTWC=UZTWC+PX6
C      PAV=0.0
C      GO TO 233
C
C      MOISTURE AVAILABLE IN EXCESS OF UZTW STORAGE.
C
C 232 UZTWC=UZTWM
C 233 ADIMC=ADIMC+PX6-PAV
C
C ****
C
C      SRF=0.0
C      SSUR=0.0
C      SIF=0.0
C      SPERC=0.0
C      SDRO=0.0
C
C      NINC=1.0+0.2*(UZFWC+PAV)
C
C      NINC=NUMBER OF TIME INCREMENTS THAT THE SIX
C      HOUR PERIOD IS DIVIDED INTO FOR FURTHER
C      SOIL-MOISTURE ACCOUNTING. NO ONE PERIOD
C      WILL EXCEED 5.0 MILLIMETERS OF UZFWC+PAV
C
C      DINC=(1.0/NINC)*0.25
C
C      DINC=LENGTH OF EACH INCREMENT IN DAYS.
C
C      PINC=PAV/NINC
C
C      PINC=AMOUNT OF AVAILABLE MOISTURE FOR EACH INCREMENT.
C      COMPUTE FREE WATER DEPLETION FRACTIONS FOR
C      THE TIME INTERVAL BEING USED-BASIC DEPLETIONS
C      ARE FOR ONE DAY
C
C      DUZ=1.0-((1.0-UZK)**DINC)
C      DLZP=1.0-((1.0-LZPK)**DINC)
C      DLZS=1.0-((1.0-LZSK)**DINC)
C
C
C
C
C      DO 240 IC=1,NINC
C
C      PAV=PINC
C      ADSUR=0.0
C      RATIO=(ADIMC-UZTWC)/LZTWM
C      ADDR0=PINC*(RATIO**2)
C      SDRO=SDRO+ADDR0*ADIMP
C
C      ADDR0 IS THE AMOUNT OF DIRECT RUNOFF FROM
C      THE AREA ADIMP-SDRO IS THE SIX HOUR SUMMATION
C      COMPUTE BASEFLOW AND KEEP TRACK OF SIX-HOUR SUM.
C
C      BF=LZFPC*DLZP
C      LZFPC=LZFPC-BF
C
C      IF (LZFPC.GT.0.0001) GO TO 234
C
C      BF=BF+LZFPC
C      LZFPC=0.0
C
C 234 SRF=$RF+BF
C      BF=LZFSC*DLZS
C      LZFSC=LZFSC-BF
C
C      IF(LZFSC.GT.0.0001) GO TO 235
C

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BF=BF+LZFSC
LZFSC=0.0
C 235 SBF=SBF+BF
C
C.....COMPUTE PERCOLATION-IF NO WATER AVAILABLE THEN SKIP
C IF((PINC+UZFWC).GT.0.01) GO TO 251
C UZFWC=UZFWC+PINC
C GO TO 249
C 251 PERCM=LZFP*DLZP+LZFSM*DLZS
PERC=PERCM*(UZFWC/UZFWM)
DEFR=1.0-((LZTWC+LZFP+LZFSC)/(LZTWM+LZFP+LZFSM))
C DEFR IS THE LOWER ZONE MOISTURE DEFICIENCY RATIO
C PERC=PERC*(1.0+ZPERC*(DEFR**REXP))
C NOTE...PERCOLATION OCCURS FROM UZFWC BEFORE PAV IS ADDED.
C IF(PERC.LT.UZFWC) GO TO 241
C PERCOLATION RATE EXCEEDS UZFWC.
C PERC=UZFWC
UZFWC=0.0
GO TO 247
C PERCOLATION RATE IS LESS THAN UZFWC.
C 241 UZFWC=UZFWC-PERC
C CHECK TO SEE IF PERCOLATION EXCEEDS LOWER ZONE DEFICIENCY.
C CHECK=LZTWC+LZFP+LZFSC+PERC-LZTWM-LZFP-LZFSM
C IF(CHECK.LE.0.0) GO TO 242
PERC=PERC-CHECK
UZFWC=UZFWC+CHECK
C 242 SPERC=SPERC+PERC
C SPERC IS THE SIX HOUR SUMMATION OF PERC
C
C.....COMPUTE INTERFLOW AND KEEP TRACK OF SIX HOUR SUM.
C NOTE...PAV HAS NOT YET BEEN ADDED.
C DEL=UZFWC*DULZ
SIF=SIF+DEL
UZFWC=UZFWC-DEL
C
C.....DISTRIB PERCOLATED WATER INTO THE LOWER ZONES
C TENSION WATER MUST BE FILLED FIRST EXCEPT FOR THE FREE AREA.
C 247 VPERC=PERC
PERC=PERC*(1.0-PFREE)
C IF((PERC+LZTWC).GT.LZTWM) GO TO 243
LZTWC=LZTWC+PERC
PERC=0.0
GO TO 244
C 243 PERC=PERC+LZTWC-LZTWM
LZTWC=LZTWM
C DISTRIBUTE PERCOLATION IN EXCESS OF TENSION
C REQUIREMENTS AMONG THE FREE WATER STORAGES.

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244 PERC=PERC+VPERC*PFREE
      IF(PERC.EQ.0.0) GO TO 245
      HPL=LZFP/(LZFP+LZFSM)
C
C      HPL IS THE RELATIVE SIZE OF THE PRIMARY STORAGE
C      AS COMPARED WITH TOTAL LOWER ZONE FREE WATER STORAGE.
C
C      RATLP=LZFP/LZFP
C      RATLS=LZFS/LZFSM
C
C      RATLP AND RATLS ARE CONTENT TO CAPACITY RATIOS, OR
C      IN OTHER WORDS, THE RELATIVE FULLNESS OF EACH STORAGE
C
C      PERCP=PERC*((HPL*2.0*(1.0-RATLP))/((1.0-RATLP)+(1.0-RATLS)))
C      PERCS=PERC-PERCP
C
C      PERCP AND PERCS ARE THE AMOUNT OF THE EXCESS
C      PERCOLATION GOING TO PRIMARY AND SUPPLEMENTAL
C      STORGES, RESPECTIVELY.
C
C      LZFS=LZFS+PERCS
C
C      IF(LZFS.LE.LZFSM) GO TO 246
C      PERCS=PERCS-LZFS+LZFSM
C      LZFS=LZFSM
C
C      246 LZFP=LZFP+(PERC-PERCS)

C.....C
C      DISTRIBUTE PAV BETWEEN UZFWC AND SURFACE RUNOFF.
C
C      245 IF(PAV.EQ.0.0) GO TO 249
C      CHECK IF PAV EXCEEDS UZFWM
C      IF((PAV+UZFWC).GT.UZFWM) GO TO 248
C      NO SURFACE RUNOFF
C      UZFWC=UZFWC+PAV
C      GO TO 249

C.....C
C      COMPUTE SURFACE RUNOFF AND KEEP TRACK OF SIX HOUR SUM
C
C      248 PAV=PAV+UZFWC-UZFWM
C      UZFWC=UZFWM
C      SSUR=SSUR+PAV*PAREA
C      ADSUR=PAV*(1.0-ADDR0/PINC)
C
C      ADSUR IS THE AMOUNT OF SURFACE RUNOFF WHICH COMES
C      FROM THAT PORTION OF ADIMP WHICH IS NOT
C      CURRENTLY GENERATING DIRECT RUNOFF. ADDR0/PINC
C      IS THE FRACTION OF ADIMP CURRENTLY GENERATING
C      DIRECT RUNOFF.

C      SSUR=SSUR+ADSUR*ADIMP
C      249 ADIMC=ADIMC+PINC-ADDR0-ADSUR
C
C      240 CONTINUE

C.....C
C      END OF INCREMENTAL DO LOOP.

C*****
C

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C COMPUTE SUMS AND ADJUST RUNOFF AMOUNTS BY THE AREA OVER
C WHICH THEY ARE GENERATED.
C EUSED=E1+E2+E3
C EUSED IS THE ET FROM PAREA WHICH IS 1.0-ADIMP-PCTIM
C SIF=SIF*PAREA
C SEPARATE CHANNEL COMPONENT OF BASEFLOW
C FROM THE NON-CHANNEL COMPONENT
C TBF=SBF*PAREA
C TBF IS TOTAL BASEFLOW
C BFCC=TBF*(1.0/(1.0+SIDE))
C BFCC IS BASEFLOW, CHANNEL COMPONENT
C BFNCC=TBF-BFCC
C BFNCC IS BASEFLOW, NON-CHANNEL COMPONENT
C .....*
C ADD TO MONTHLY SUMS.
C SINTFT=SINTFT+SIF
C SGWFT=SGWFT+BFCC
C SRFCHT=SRECHT+BFNCC
C SR0ST=SR0ST+SSUR
C SR0DT=SR0DT+SDR0
C
C COMPUTE TOTAL CHANNEL INFLOW FOR THE SIX-HOUR PERIOD.
C TCI=ROIMP+SDR0+SSUR+SIF+BFCC
C COMPUTE E4-ET FROM STREAM SURFACES AND RIPARIAN VEGETATION.
C E4=EDMND*WATSF+(EDMND-EUSED)*SARRA
C SUBTRACT E4 FROM CHANNEL INFLOW
C TCI=TCI-E4
C IF(TCI.GE.0.0) GO TO 250
C E4=E4+TCI
C TCI=0.0
C
C COMPUTE TOTAL EVAPOTRANSPIRATION-TET
C 250 FUSED=FUSED*PAREA
C TET=EUSED+E5+E4
C SFTT=SETT+TET
C .....*
C RO(IRG,IP6,IDA) = TCI
C .....*
C SR0T=SR0T+TCI
C
C PRINT SIX-HOUR ACCOUNTING VALUES IF REQUESTED.
C IF(IPRINT.EQ.1) PRINT 903,IDA,IP6,UZTWC,UZFWC,LZTWC,LZFSC,LZPFC,AD
C 1IMC,SPERC,ROIMP,SDR0,SSUR,SIF,BFCC,TCI,EDMND,TET,PX6
C 903 FORMAT(1H .2I3.6F7.1,7F8.2,3F8.1)
C IF((IDA.EQ.ID2).AND.(IP6.EQ.IP2)) GO TO 270
C IP6=IP6+1

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```
C      IF(IP6.LE.4) GO TO 205
C
C      IP6=1
C      IDA=IDA+1
C      GO TO 205
C
C*****END OF SIX HOUR AND DAY LOOP*****
C
C*****END OF SIX HOUR AND DAY LOOP*****
C
C      END OF SIX HOUR AND DAY LOOP
C
C*****END OF SIX HOUR AND DAY LOOP*****
C
C      270 IF(IRG.NE.NGAGES) GO TO 271
C          IF((IPRINT.EQ.1).AND.(ICOUNT.LT.8)) ICOUNT=ICOUNT+1
C      271 IPRINT=0
C
C      COMPUTE MONTHLY WATER BALANCE FOR AREAL SOIL MOISTURE ACCOUNTING.
C
C      BAL(IRG)=(UZTWC+UZFWC+LZTWC+LZFPC+LZFSC-UZTWC1-UZFWC1-LZTWC1-LZFPC
C      11-LZFSC1)*PAREA+(ADIMC-ADIMC1)*ADIMP+SR0T+SRECHT+SETT-SPRT
C
C*****SL(IRG.1)=SR0T
C*****SL(IRG.2)=SIMPVT
C*****SL(IRG.3)=SR0DT
C*****SL(IRG.4)=SR0ST
C*****SL(IRG.5)=SINTFT
C*****SL(IRG.6)=SGWFT
C*****SL(IRG.7)=SRECHT
C*****SL(IRG.8)=SPRT
C*****SL(IRG.9)=SPET
C*****SL(IRG.10)=SETT
C      VL(IRG.1)=UZTWC
C      VL(IRG.2)=UZFWC
C      VL(IRG.3)=LZTWC
C      VL(IRG.4)=LZFPC
C      VL(IRG.6)=ADIMC
C
C      RETURN
C
C      END
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DICTIONARY FOR SUBROUTINE LAND...NWS.HRL. VERSION 9/11/75

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**SYMBOL ... EXPLANATION**

|     |   |
|-----|---|
| F   | ... MEAN SEASONAL POT-EVAP CURVE ARRAY                    |
| I   | ... INDEX   |
| BF  | ... BASE FLOW   |
| EP  | ... DAILY EVAPORATION                                     |
| E1  | ... EVAP FROM UPPER ZONE TENSION WATER                    |
| E2  | ... EVAP FROM UPPER ZONE FREE WATER                       |
| E3  | ... EVAP FROM LOWER ZONE TENSION WATER                    |
| E4  | ... EVAP FROM STREAM SURFACES AND RIPARIAN VEGETATION     |
| E5  | ... EVAP FROM ADDITIONAL IMPERVIOUS AREA                  |
| IC  | ... INDEX   |
| PF  | ... POTENTIAL EVAPORATION ARRAY                           |
| PL  | ... INITIAL PARAMETER VALUE ARRAY                         |
| PX  | ... PRECIPITATION ARRAY                                   |
| RO  | ... RUNOFF ARRAY  |
| SL  | ... ARRAY CONTAINING MONTHLY TOTALS OF VARIOUS COMPONENTS |
| TA  | ... VARIABLE IN COMMON                                    |
| VL  | ... ARRAY CONTAINING SOIL MOISTURE STORAGE VOLUMES        |
| AID | ... AREA IDENTIFICATION                                   |
| BAL | ... WATER BALANCE   |
| DEL | ... INCREMENTAL VOLUME OF WATER                           |
| DUZ | ... UPPER ZONE FREE WATER DEPLETION COEFFICIENT           |
| EFC | ... EVAP ADJUSTMENT FACTOR                                |
| HPL | ... RATIO LZFPM/(LZFPM + LZFSM)                           |
| IDA | ... DAY INDEX   |
| ID1 | ... FIRST DAY   |
| ID2 | ... LAST DAY  |
| IPT | ... VARIABLE IN COMMON BLOCK ONLY                         |
| IP1 | ... FIRST PERIOD OF FIRST DAY                             |
| IP2 | ... LAST PERIOD OF LAST DAY                               |
| IP6 | ... SIX HOUR PERIOD INDEX                                 |
| IRG | ... INDEX   |
| NQ6 | ... VARIABLE IN COMMON BLOCK ONLY                         |
| PAV | ... MOISTURE IN EXCESS OF UZTW REQUIREMENTS               |
| PEG | ... POTENTIAL EVAPORATION OPTION VARIABLE                 |
| PX6 | ... SIX-HOUR PRECIPITATION                                |
| RFD | ... RESIDUAL EVAPORATION DEMAND                           |
| SRF | ... SUPPLEMENTAL BASE FLOW                                |
| SIF | ... INTERFLOW   |
| SOF | ... VARIABLE IN COMMON ONLY                               |
| SSF | ... VARIABLE IN COMMON ONLY                               |
| TRF | ... TOTAL BASE FLOW                                       |
| TCI | ... TOTAL CHANNEL INFLOW                                  |
| TET | ... TOTAL EVAPOTRANSPIRATION                              |
| UZK | ... UPPER ZONE DRAINAGE PARAMETER                         |
| YR1 | ... FIRST YEAR  |
| YR2 | ... LAST YEAR   |

|      |  |
|------|--|
| AREA | ... AREA NAME  |
| BFCC | ... BASE FLOW CHANNEL COMPONENT  |
| DEFR | ... LOWER ZONE MOISTURE DEFICIENCY RATIO                                     |
| DINC | ... LENGTH OF SOIL MOISTURE ACCOUNTING TIME INTERVAL IN DAYS                 |
| DLZP | ... LOWER ZONE PRIMARY STORAGE DEPLETION COEFFICIENT                         |
| DLZS | ... LOWER ZONE SUPPLEMENTAL STORAGE DEPLETION COEFFICIENT                    |
| EFCT | ... EVAPORATION ADJUSTMENT FACTOR  |
| FPID | ... VARIABLE IN COMMON BLOCK --- FLOW POINT I.D.                             |
| IGPE | ... POTENTIAL EVAP DATA OPTION VARIABLE                                      |
| LAND | ... SUBROUTINE NAME  |
| LAST | ... VARIABLE IN COMMON BLOCK ONLY  |
| LZPK | ... LOWER ZONE PRIMARY STORAGE DRAINAGE PARAMETER                            |
| LZSK | ... LOWER ZONE SUPPLEMENTAL STORAGE DRAINAGE PARAMETER                       |
| MOIN | ... VARIABLE IN COMMON ONLY  |
| MOSM | ... MONTHS FOR WHICH A DETAILED SOIL MOISTURE OUTPUT IS REQUESTED            |
| NINC | ... NUMBER OF INTERVALS IN ONE 6-HR PERIOD USED FOR SOIL MOISTURE ACCOUNTING |
| NPTS | ... VARIABLE IN COMMON ONLY  |
| NQ24 | ... VARIABLE IN COMMON ONLY --- NUMBER OF DAILY FLOW TIME SERIES             |
| NTAG | ... VARIABLE IN COMMON ONLY--NUMBER OF AIR TEMPERATURE TIME SERIES           |
| NWEG | ... VARIABLE IN COMMON ONLY --NUMBER OF WATER EQUIV. TIME SERIES             |
| OFW6 | ... VARIABLE IN COMMON ONLY  |
| PEID | ... VARIABLE IN COMMON ONLY  |
| PERC | ... PERCOLATION RATE   |
| PINC | ... AMOUNT OF AVAILABLE MOISTURE FOR EACH INCREMENT                          |
| PLOT | ... VARIABLE IN COMMON ONLY  |
| PXID | ... VARIABLE IN COMMON ONLY  |
| Q6ID | ... VARIABLE IN COMMON ONLY  |
| REXP | ... EXPONENT IN PERCOLATION EQUATION   |
| SDRO | ... 6-HR SUMMATION OF DIRECT RUNOFF  |
| SETT | ... MONTHLY SUMMATION OF EVAPOTRANSPIRATION                                  |
| SFW6 | ... VARIABLE IN COMMON ONLY  |
| SIDE | ... PARAMETER SEPARATING CHANNEL AND NON-CHANNEL INFLOW                      |
| SNOW | ... SNOW OPTION VARIABLE   |
| SPFT | ... MONTHLY SUM OF POTENTIAL EVAPORATION                                     |
| SPRT | ... MONTHLY SUM OF PRECIPITATION   |
| SROT | ... MONTHLY SUM OF RUNOFF OR TOTAL CHANNEL INFLOW                            |
| SSUR | ... MONTHLY SUM OF SURFACE RUNOFF  |
| STAT | ... VARIABLE IN COMMON ONLY  |
| STDA | ... VARIABLE IN COMMON ONLY  |
| STP6 | ... VARIABLE IN COMMON ONLY  |
| UFW6 | ... VARIABLE IN COMMON ONLY  |
| YFAR | ... CURRENT YEAR   |
| YRIN | ... VARIABLE IN COMMON ONLY  |

ADDRO . . . DIRECT RUNOFF FROM AREA ADIMP  
 ADMIC . . . ADDITIONAL IMPERVIOUS AREA STORAGE  
 ADIMP . . . ADDITIONAL IMPERVIOUS AREA  
 ADSUR . . . SURFACE RUNOFF FROM PORTION OF ADIMP NOT PRODUCING ADDRO  
 ANAME . . . AREA NAME  
 BASIN . . . VARIABLE IN COMMON ONLY  
 BFNCC . . . BASE FLOW-NONCHANNEL COMPONENT  
 CHECK . . . A PERCOLATION RATE CHECK  
 COVER . . . SNOW COVER  
 CTEST . . . VARIABLE IN COMMON ONLY  
 EDMND . . . EVAPORATION DEMAND FOR SIX HOURS  
 EUSED . . . EVAPOTRANSPIRATION FROM PAREA=1.0-ADIMP-PCTIM  
 INFRO . . . VARIABLE IN COMMON ONLY  
 IQ6IN . . . VARIABLE IN COMMON ONLY  
 LINEP . . . VARIABLE IN COMMON ONLY  
 LZFPC . . . LOWER ZONE PRIMARY FREE WATER STORAGE CONTENTS  
 LZFPM . . . LOWER ZONE PRIMARY FREE WATER STORAGE MAXIMUM  
 LZFSC . . . LOWER ZONE SUPPLEMENTAL FREE WATER STORAGE CONTENTS  
 LZFSM . . . LOWER ZONE SUPPLEMENTAL FREE WATER STORAGE MAXIMUM  
 LZTWC . . . LOWER ZONE TENSION WATER STORAGE CONTENTS  
 LZTWM . . . LOWER ZONE TENSION WATER STORAGE MAXIMUM  
 MONTH . . . CURRENT MONTH  
 NPEGS . . . VARIABLE IN COMMON ONLY  
 ORSER . . . VARIABLE IN COMMON ONLY  
 OFW24 . . . VARIABLE IN COMMON ONLY  
 PAREA . . . PAREA=1.0-ADIMP-PCTIM  
 PCTIM . . . PERCENT OF AREA THAT IS IMPERVIOUS  
 PFADJ . . . POTENTIAL EVAPORATION ADJUSTMENT FACTOR  
 PERCM . . . DISCHARGE FROM LOWER ZONE  
 PERCP . . . AMOUNT OF PERCOLATED WATER TO LOWER ZONE PRIMARY STORAGE  
 PERCS . . . AMOUNT OF PERCOLATED WATER TO LOWER ZONE SUPPLEMENTAL STORAGE  
 PFREF . . . PERCENTAGE OF PERCOLATED WATER TO LOWER ZONE FREE WATER STORAGE  
 PPADJ . . . PRECIPITATION ADJUSTMENT FACTOR  
 PTEST . . . VARIABLE IN COMMON ONLY  
 PXADJ . . . VARIABLE IN COMMON ONLY  
 Q24ID . . . VARIABLE IN COMMON ONLY  
 RATIO . . . RATIO (ADIMC-UZTWC)/LZTWM  
 RATLP . . . LOWER ZONE PRIMARY CONTENTS TO CAPACITY RATIO  
 RATLS . . . LOWER ZONE SUPPLEMENTAL CONTENTS TO CAPACITY RATIO  
 RATLZ . . . TOTAL LOWER ZONE STORAGE CONTENTS TO CAPACITY RATIO  
 ROIMP . . . RUNOFF FROM IMPERVIOUS AREA  
 ROUTE . . . VARIABLE IN COMMON ONLY  
 RSFRV . . . LOWER ZONE FREE WATER THAT IS UNAVAIL TO MEET LZTW REQUIREMENTS  
 SARRA . . . SARRA=SARVA-PCTIM  
 SARVA . . . PERCENT OF AREA IN STREAM AND RIPARIAN VEGETATION  
 SAVED . . . VOLUME OF LOWER ZONE FREE WATER NOT AVAILABLE FOR LZTW  
 SGWFT . . . MONTHLY SUM OF BASE FLOW REACHING THE CHANNEL  
 SIXIN . . . VARIABLE IN COMMON ONLY  
 SNOWA . . . ARRAY CONTAINING INDICATORS FOR VALID AIR-TEMP DATA FOR EACH MONTH  
 SPERC . . . 6-HR SUMMATION OF PERC  
 SRDT . . . SUMMATION OF DIRECT RUNOFF  
 SROST . . . SUMMATION OF SURFACE RUNOFF  
 STORE . . . VARIABLE IN COMMON ONLY  
 UZFWC . . . UPPER ZONE FREE WATER CONTENTS  
 UZFWM . . . UPPER ZONE FREE WATER MAXIMUM  
 UZRAT . . . UPPER ZONE CONTENTS TO CAPACITY RATIO  
 UZTWC . . . UPPER ZONE TENSION WATER CONTENTS  
 UZTWM . . . UPPER ZONE TENSION WATER MAXIMUM  
 VPERC . . . TEMPORARY STORAGE VARIABLE FOR PERC  
 WATSF . . . WATER SURFACE AREA  
 ZPERC . . . PERCOLATION PARAMETER

ADIMC1 ... INITIAL CONTENTS OF ADIMC  
COMPAR ... VARIABLE IN COMMON ONLY  
EPDIST ... DISTRIBUTION OF DAILY POTENTIAL EVAP  
FPNAME ... VARIABLE IN COMMON ONLY  
FSFLOW ... VARIABLE IN COMMON ONLY  
ICOUNT ... INDEX  
IPRINT ... PRINT OPTION VARIABLE  
I024IN ... VARIABLE IN COMMON ONLY  
IYEAR1 ... VARIABLE IN COMMON ONLY  
LZFPC1 ... INITIAL VALUE OF LZFPC  
LZFSC1 ... INITIAL VALUE OF LZFSC  
LZTWC1 ... INITIAL VALUE OF LZTWC  
METRIC ... VARIABLE IN COMMON ONLY  
NGAGES ... NUMBER OF RAIN GAGES  
NPTSUP ... VARIABLE IN COMMON ONLY  
PLOTMX ... VARIABLE IN COMMON ONLY  
PLT6HR ... VARIABLE IN COMMON ONLY  
RATLZT ... LOWER ZONE TENSION WATER STORAGE CONTENTS TO CAPACITY RATIO  
SAVEFW ... VARIABLE IN COMMON ONLY  
SIMPV1 ... SUMMATION OF IMPERVIOUS AREA RUNOFF  
SINTFT ... MONTHLY SUMMATION OF INTERFLOW  
SRECHT ... MONTHLY SUMMATION OF CHANNEL COMPONENT OF BASE FLOW  
UPFWID ... VARIABLE IN COMMON ONLY  
UZFWC1 ... INITIAL VALUE OF UZFWC  
UZTWC1 ... INITIAL VALUE OF UZTWC







(Continued from inside front cover)

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